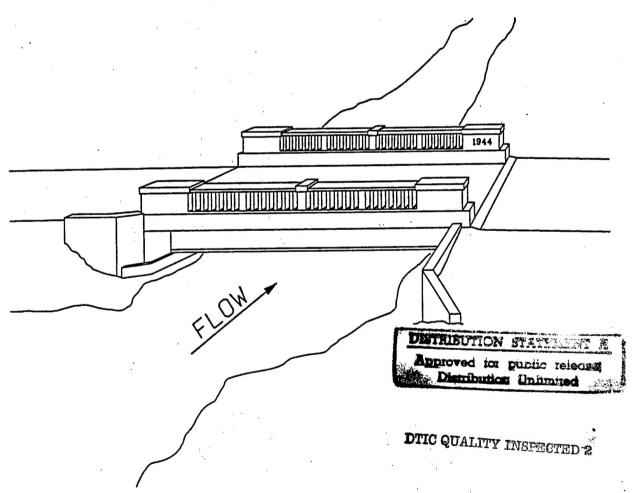


## Bridge Inspection Program

**FY 93 Routine Inspections** 



October 1993

New England Division

19980206 113

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bridges, inspection, Franklin Falls, Birch Hill, West Hill,

18. SECURITY CLASSIFICATION OF THIS PAGE

Unclassified

Everett Lake, Otter Lake, Colebrook, Knightville

Thomaston, Northfield Brook, Black Rock, Hop Brook, Tully Lake, 16. PRICE CODE

19. SECURITY CLASSIFICATION OF ABSTRACT Unclassified Standard Form 298 (Rev. 2-89) Presented by AMS SEE, 239-18 298-102

20. LIMITATION OF ABSTRACT

Unclassified

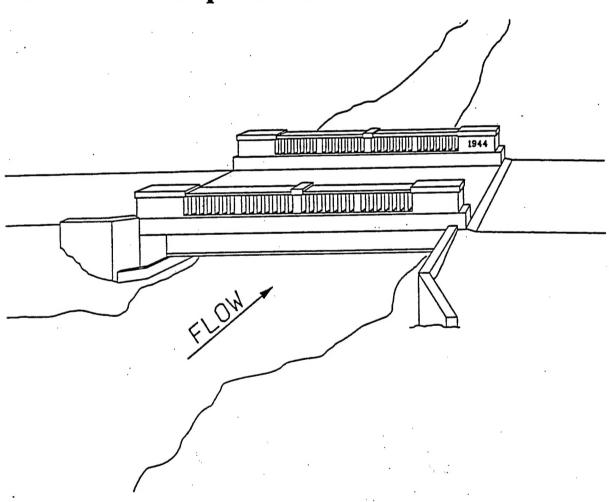
OF REPORT

17. SECURITY CLASSIFICATION



# Bridge Inspection Program

**FY 93 Routine Inspections** 



October 1993
New England Division

#### BRIDGE INSPECTION PROGRAM FY 93 ROUTINE INSPECTIONS NEW ENGLAND DIVISION

OCTOBER 1993

DEPARTMENT OF THE ARMY
NEW ENGLAND DIVISION, CORPS OF ENGINEERS
WALTHAM, MASSACHUSETTS

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Everett Lake --- Choate Brook

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## NEW ENGLAND DIVISION FY 93 ROUTINE BRIDGE INSPECTION PROGRAM

#### PURPOSE AND SCOPE

The purpose of the routine bridge inspections is to inspect the physical condition of the structures and to verify and update the findings and evaluations reported in the last in-depth and routine inspection. All previously detected areas of structural distress or operational inadequacies were reevaluated and any new deficiencies documented with the overall goal being to increase the useful life of the structures and to ensure the continued safety of the bridge users.

#### AUTHORITY

The basis for the inspections is contained in ER 1110-2-111 "Periodic Safety Inspection and Continuing Evaluation of United States Army Corps of Engineers Bridges."

#### INSPECTION PROCEDURE

The overall inspections were performed in accordance with AASHTO's 1983 "Manual for Maintenance Inspection of Bridges", the Department of Transportation's "Bridge Inspector's Training Manual 90" (1990 edition) and all applicable sections of ER 1110-2-111. The inspection program was carried out under the direct supervision of a licensed Professional Engineer. The most recent in-depth inspection reports were thoroughly reviewed by inspection personnel prior to and during the field inspections.

The underside of all smaller Reservoir Area bridges were accessed using a ladder, waders and a small boat, or some combination thereof, as required.

During all inspections, all pertinent safety equipment was utilized and all pertinent safety procedures were followed.

#### REPORTING PROCEDURE

For each bridge, an overall summary has been prepared. Included are the vehicle ratings, evaluation of each structural component, and overall structural evaluation, all compared with those from all previous inspections. Also included are the previous recommended remedial repairs, the status of these recommendations and any new recommendations and/or comments based on the current inspections.

Field-completed checklists for each bridge are the Standard Structures Inspection Field Report and the Scour Checklist (an NED devised checklist based on recent Federal Highway Administration guidelines to more precisely address any potential or active scour-related problems).

#### BRIDGES INSPECTED

For the 1993 Interim Bridge Inspection Program, 22 bridges were inspected as indicated herein. Bridges inspected, projects, 1993 and 1991 condition ratings, inspection dates, estimated rehabilitation costs, rehabilitation priorities (see below) with temporary posting required, and degree of existing scour (see below) are summarized on next page:

Rehab. Priority (Posting, if necessary, in tons required)

- 1. Bridge currently cannot tolerate present traffic/loads. Prompt remedial measures are required. Bridge should be posted and restricted as indicated until corrective measures can be accomplished.
  - 2. Major items require rehabilitation. Minimum adequacy to tolerate present traffic/loads. Further deterioration may cause priority 1.
  - 3. Minor items require rehabilitation to maintain condition.

#### Scour

- 1 Major Scour Activity/Potential
- 2 Moderate Scour Activity/Potential
- 3 Minimal or No Scour Activity/Potential

Project/Bridge Rehab. Scour		Condition	Da	Est.Rehab.			
		Rating	Ir	spected	Cost	(K)	
Priority		1991 1993	3				(see
below)							•
FRANKLIN FALLS							
1. Lower Mill Brook	4	4	7/14/93			1*	3**
2. Upper Mill Brook	4	4	7/15/93			1*	2**
3. Knox Brook	4	9	7/14/93			3	2
4. Blake Brook	7	7	7/14/93			3	3
5. Smith River	5	5	7/14/93	3 180 <u>+</u>		1	(5) *2**
BIRCH HILL		_				_	
6. Middle Road	7	7	6/25/93			3	3**
<ol> <li>New Boston Road</li> </ol>	7	8	6/24/93			3	3
<ol><li>8. Burgess Road</li></ol>	7	7	6/24/93			2	3
9. Old Route 202	7	7	6/24/93			3	2**
10. Goodnow Road	7	7	6/25/93	5.0		3	2**
WEST HILL							
11. West Hill Road	5	7	9/08/93	91.5		2	2
THOMASTON							
12. Leadmine Brook	8	8	8/24/93	0.0		3	2
NORTHFIELD BROOK							
13. Old Rt.254 (upper)	8	7	8/24/93			3	2
14. Old Rt.254 (lower)	8	7	8/24/93	0.0		3	3
BLACK ROCK							
15. Old Northfield Rd.	8	8	8/24/93	31.0		3	3
HOP BROOK							
16. Old Route 63	5	, <b>7</b>	8/24/93	0.5		3	3
TULLY LAKE							-2
17. Doane Hill Road	7	, 7	6/25/93	3 25.0		3	3
EVERETT LAKE							
18. Choate Brook	4	7	9/09/93	0.0		3	2**
OTTER BROOK							
<pre>19. Rec Area (Exit)</pre>	6	6	8/18/93			2	2**
20. Rec Area (Entran.)	7	7	8/18/93	32.0		2	2**
COLEBROOK							,
21. Old Route 8	7	8	8/25/93	0.0		3	3
KNIGHTVILLE							_
22. Indian Hollow	7	7	8/25/93	3 5.0		3	3

### LEGEND

See overall assessment.
Scour analysis performed. \*\*

#### OVERALL ASSESSMENT

During FY93, only reservoir area bridges (no spillway bridges) were inspected. Overall, the condition of the bridges inspected ranged from good to fair to poor, with overall condition ratings and rehabilitation priorities as listed above.

#### REHAB PRIORITY 1

Bridges that were assessed a rehab priority of 1, with corresponding reduction in capacity are as follows:

<u>Project</u>	<u>Bridge</u>	<u>Temporary Posting</u>
Franklin Falls	Upper Mill Brook	close
Franklin Falls	Lower Mill Brook	6
Franklin Falls	Smith River	5

These three bridges have been given the highest priority, with recommendations listed herein to be expeditiously carried out. Until these bridges have been rehabilitated as indicated, the above posting for each bridge shall be strictly adhered to.

Operation Directorate has made an assessment of the future intended usage of the bridges to determine what level of rehabilitation, if any, is required. Based on their decision, the following has been recommended by Engineering Directorate with concurrence from Operations Directorate:

- 1. Upper Mill Brook will be permanently closed to vehicular traffic by installation of permanent barriers on the east and west approaches.
- 2. Lower Mill Brook will be rehabilitated as recommended with design and construction budgeted for FY 94.
- 3. Smith River Bridge will be immediately posted for a 5 tons weight rating and 10 mph speed limit in order to limit usage to small truck traffic. Interim inspections will be performed on the structure at six month intervals to determine if further deterioration requires further reduction of capacity or complete closure.

#### FRACTURE CRITICALITY

Of the bridges inspected, only Old Route 8 Bridge falls into the fracture critical category. It is a two truss, simple span, through truss, steel structure with built up members and riveted connections. Some of the rivets have been replaced with high strength bolts. Because of its location (within the reservoir), it is subjected to very minimal traffic, in general, and therefore, minimal truck traffic. Because of this low traffic volume and the overall good condition of the structure, no additional testing is required and continued two year inspection

intervals is considered sufficient.

#### SCOUR

The FY 93 routine inspections also include a scour checklist (an NED devised checklist based on recent FHWA guidelines) which was encorporated to better define any active or potential scour related problems.

Scour problems have been noted at the following bridges and listed in order of relative severity:

```
Otter Brook ---- Recreation Area (Exit)
Otter Brook ---- Recreation Area (Entrance)
Birch Hill ---- Goodnow Road
Thomaston ---- Leadmine Brook
Birch Hill ---- Old Route 202
Everett Lake --- Choate Brook
```

Remedial measures have been listed in the recommendation section of the text for each structure. None of these conditions are considered to be of such criticality that immediate action is warranted. Repairs should be performed in a timely manner through normal budgetary procedures and priorities, and continued monitoring of scour conditions should be performed during all future inspections.

FY 93 scour assessments, both Hydrologic/Hydraulic and Geotechnical were performed on bridges at Everett Lake (Choate Brook) and Birch Hill (Goodnow Road, Middle Road, Old Route 202) with the results encorporated into the inspection reports. This makes a total of nine bridges as indicated in the summary above, in the inventory, which have been assessed in this manner for scour criticality. Based on the extremely low probability of failure from scour, it is recommended that not further in-depth Hydrologic/Hydraulic scour assessments be performed, on any bridges, unless recommended as a result of specific findings during future inspections.

## FRANKLIN FALLS DAM LOWER MILL BROOK BRIDGE, HILL, NH FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 14 July 93

DATE OF PREVIOUS INSPECTIONS: In-Depth, 9, 10 July 85

Routine, 17 July 87 Routine, 28 April 89 Routine, 15 May 91

RATING (T=TONS)

Type Inventory Operating Comments

H15 6.8T 15.2T No change in ratings

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway and Railings: Overall condition is poor (condition 4).

The gravel wearing surface is well

graded. Vegetative growth and debris are evident on the deck and approaches. The growth includes one 4- and one 6- inch tree on the shoulder. The approaches are in good condition. The transitions are good on both approaches. The load rating is not posted. There are no drains on the deck. The steel pipe bridge and approach railings are missing large sections and are heavily

the streambed.

B. Fascias & Curbs: The fascias and curbs are in fair

corroded.

condition. There is some concrete spalling evident on the exterior of the west fascia. There is minor

There is a steep drop to

efflorescence from the concrete over and

around the exterior steel beams.

C. Underside of Deck: The overall condition of the underside

of the concrete deck is fair (condition 5). One moderate spall was noted on the

inside southwest corner of the

exterior concrete arch beam. The area of

this spall was described in the 91 investigation to be approximately four

square feet. There has been no

significant increase in size since that observation. Minor efflorescence was observed on the underside of the deck.

Minor cracks were observed in the

concrete arch between the two exterior beams. There is some minor surface corrosion of the exposed steel beams and tie rods. Graffiti is evident.

D. Wingwalls/Abutments:

The condition of the abutments is fair (condition 5). The north and south abutments contain minor hairline cracks and efflorescence. There is an approximately 36-inch crack at the junction of the north abutment and west bridge deck which intersects the west wingwall.

The overall condition of the channel training walls is fair (condition 5). The northwest channel training wall has minor cracks. The southwest channel training wall is of stone rubble masonry and exhibits minor effects of erosion. The mortar is eroded but there is no evidence of rock loss or movement.

The wingwalls are is fair condition. All wingwalls have minor cracks, spalls and efflorescence. Wingwall drainage pipes are covered by vegetation. No catch basin was observed. Several full-length vertical cracks were found on northeast and northwest wingwalls.

E. Channel:

The channel shows no sign of scour. There are no obstructions or debris in the channel.

CONDITION RATINGS:

In-depth, 1985: 7
Routine, 1989: 5
Routine, 1991: 4
Routine, 1993: 4

#### Status of Previous Recommendations

Item Current Status

1. Install steel beam guardrail. Not Done

2. Repair cracks at approaches and NW corner of deck.

3. Patch spalled concrete and repair sidewalk.

- 4. Sandblast and paint steel beams. Not Done
- 5. Regrade roadbed.

Done

#### Revised Recommendations

1. Install steel beam guardrail.

Estimated cost \$30,000.

2. Repair cracks on the wingwalls of the north abutments and crack at NW corner of the deck. Patch mortar on the southwest training wall.

Estimated cost \$5,000.

3. Patch all areas of spalled concrete on fascia and curbs and the inside southwest corner of the exterior arch beam. Repair sidewalk.

Estimated cost \$10,000.

- Sandblast and paint exposed steel surfaces. Estimated cost \$10,000.
- 5. Post load rating on approaches.

Estimated cost \$500.

Total cost \$55,500.

STRUCTURES INSPECTION FIELD REPORT Franklin Falls Down Lower Will Brook Bridg-

G., Hill	ИН		bridge dept	t. no.	8-structu	re no. EDN 4 33 10004		te inspected July 73
2-dist. 104-highway	system 4	22-owner CO	E	27-year b । ५।८		106-year rebuilt	11-mile	epoint CCCO
43-structure type Single span roll	led brambridge	e w/ reinforce	ed rence	quality co te deck	ontrol eng	ineer Nick Fabe	s	
77 facility oderiod	Access Road-1			leam lea	luei	Joseph Col.	ecci	
06-features intersected	Needle Sh	op Breck		team m	embers Walsk	n, M. Deschenes,	, L. Bo	nechanor
item 58 DECK  1. Wearing Surface 2. Deck-Condition 3. Stay in Place Form 4. Curbs 5. Median 6. Sidewalks 7. Parapet 8. Railing 9. Anti Missile Fence 10. Drains 11. Lighting Standard 12. Utilities 13. Deck Joints 14. Approach Settlem	5 Z # Z # Z # Z Z Z \	1. Bearin 2. Stringe 3. Diaphr 4. Girder 5. Floor I 6. Trusse 7. Rivets 8. Welds 9. Collisi 10. Load I 11. Memb 12. Load I 13. Paint-	ragms s or Beams Beams or Bolts on Damage Deflection oer Alignmen Vibration Epoxy Painted Clearance	t ft -	5 Z972222220@2102   S	item 60 SUBSTRUCTURE 1. Abutments a-Wings b-Backw c-Bridge d-Breast e-Footing f-Piles g-Erosio h-Settler 2. Piers or Bents a-Caps b-Colum c-Web d-Footin e-Piles f-Scour g-Settler 3. Collision Dama 4. Hydraulic-Aded	all Seats wall gs n ment g	D WZZYZZZ POZYYZZZ
Actual Posting	H 3 3S2	Single	-	Overhe	ad Signs yes	(attached to bridge) no		•
Recommended Posting From Rating Book		6.8		<ol> <li>Weld</li> <li>Bolts</li> </ol>		N		
SIGNS IN PLACE Y or N	at bridge	advar N		3. Con	dition	N		
LEGIBILITY				Item93	b U/W	Inspection Date:		
ITEM 61-channel are nannel scour 2. embankment ero 3. fender system 4. spur dikes & jettie	8 5. rip sion 7 6. effe	rap or slope pectiveness	paving N	1. br 2. tra 3. ap	idge raili ansitions	guardrail 0		ndition 3 3 N

PROJECT: Franklin Falls Dam
NAME: Lower Mill Brook Bridge
LOCATION: Hill NH

## BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	425
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	yes_
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructure with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway openings.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	425 70 70 925
3. Are any characteristics of an aggressive stream or waterway present?	no·
<ul> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	no no no no no no no
4. Is the bridge located on a stream reach with any adverse flow characteristics?	no
<ul> <li>a. Crossing near stream confluence.</li> <li>b. Crossing of tributary stream near confluence with larger streams.</li> <li>c. Crossing on sharp bend in stream.</li> <li>d. Location on alluvial fan.</li> <li>e. Other.</li> </ul>	
5. Other comments or observations.	yes
Minor erosion (mortan scour) on southwest channel traini	ng wall.

#### FRANKLIN FALLS DAM UPPER MILL BROOK BRIDGE, HILL, NH FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 15 July 93

DATE OF PREVIOUS INSPECTIONS: In-Depth, 9, 10 April 85

Routine, 17 July 87 Routine, 28 April 89 13 May 91

Routine,

#### RATING (T=TONS)

Type Inventory Operating Comments

10.7T H15 7.2T No change in ratings

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway and Railings: Overall condition is poor (condition 4).

The gravel wearing surface is very rutted. There are deep (3 inch) depressions evident at several spots on the deck. The depth of gravel wearing surface on the concrete deck is indeterminate and it is not possible to determine whether the top of the deck is damaged. Vegetative growth and debris are evident on the deck and approaches. A 10-inch tree grows on the approach near the southwestern retaining wall. There are no drains on the deck. The steel pipe bridge and approach railings are heavily rusted with large sections missing. There is a steep drop to the streambed.

The eastern approach is in poor condition with excessive settlement on the north edge of the road where the embankment and approach retaining wall are slumping. Three Jersey barriers have been placed along the north edge of the road to steer vehicles away from the failing embankment. is a sharp transition at the east approach and a steep drop to the streambed below. The western approach is similarly rutted but the transition is good. The load rating is not posted at either approach.

B. Fascias & Curbs: The fascia is in fair condition B. Fascias & Curbs:

The fascia is in fair condition (condition 5). The curbs are hidden by a thick gravel wearing surface. There is moldy growth along the exterior edge of the steel beams.

C. Underside of Deck:

The overall condition of the underside of the concrete deck is good (condition 7). There are no spalls or cracks, but there is some efflorescence. There is some minor surface corrosion of the exposed steel beams and tie rods.

D. Wingwalls/Abutments:

The condition of both abutments is good (condition 7). The east and west abutments contain cold joints.

Minor efflorescence is evident. The western abutment has 24 to 28 inches of moderate scour under an apron of the same width. The eastern abutment is slightly undermined to a depth of 16 inches under a solid apron of approximately 3 feet.

The southwest wingwall has minor cracks and efflorescence. The northwest wingwall is very overgrown by trees and other vegetation. Its general appearance is the same as that of the southwest wall. There is a 5 foot (full length), 1/2 inch wide vertical crack halfway along the wall. The southeast wingwall is covered with vegetative growth. There is some minor efflorescence. There is a full length, full depth (3 inch), 1-inch wide vertical crack midway along the wall. The northeast wingwall embankment is undercut by scour. There is a full length, full depth (3 inch), 1 1/4 inch wide crack at the 1/3 point. There is a full length, full depth, 2-inch wide crack halfway along the wall. The FY 91 bridge inspection contains diagrams detailing the site. The horizontal distance along crack 2 was measured to be 5 7/8 inches in the 91 report vs 5 3/4 inches in 93. A full length, full depth, 4-inch wide crack is located at the 2/3 point along the wall. The wall has rotated outward from the bank.

#### E. Channel:

There is an 8-inch diameter corroded cast iron pipe crossing the upstream side of the streambed. There are numerous boulders and cobbles throughout the channel. A bend in the streambed downstream of the bridge is causing eddies which are undermining the east embankment.

#### CONDITION RATINGS:

In-depth, 1985: 7
Routine, 1989: 5
Routine, 1991: 4
Routine, 1993: 4

#### Status of Previous Recommendations

Item

Current Status

1. Temporarily close bridge and extend barriers at north and south ends.

Not Done

2. Complete scour analysis.

Done

#### Revised Recommendations

The hydrologic and hydraulic assessment of Upper Mill Brook Bridge completed in 1992 recognizes a need to repair the stone and mortar aprons surrounding the bridge abutments. It also recommends that a 100 foot stone revetment which would vary in height from 5 to 15 feet be placed along the steambed to control bank erosion. The revetment would consist of 2 to 3 feet of stone protection underlain by 1 to 1.5 feet of stone bedding. Granular fill will be needed to fill eroded areas behind the revetment. A small stone dam downstream from the bridge which could be altering the direction of stream flow may need to be removed. This would require rental of a crane for a few hours to remove the stone.

Total cost \$40,000

The report notes that conditions at the bridge are severe and the cost of repairs high. It recommends closure of the bridge by installing permanent barriers on east and west approaches allowing only pedestrian and bicycle traffic. (A park gate presently exists on the east approach road only.)

Total cost (40 feet of Jersey barrier) \$500

STRUCTURES INSPECTION FIELD REPORT

Franklin Falls Dam

Opper Mill Breck Bridge

C <sub>k</sub>	H:11 NA			bridge dep	t. по.	8-structu CEP	re no. NED NH 32 10005	90-date inspected
2-dist.	104-highway sys	tem 4	22-owner COE	Ξ	27-year		106-year rebuilt	11-milepoint
43-structure			einforced a	rencrete a	quality c	ontrol eng	ineer Nick Foobes	
07-facility ca	reind	id-Reneat			team le	ader	Joseph Coluc	ci
06-features	intersected N ac	dle Shop	Brook		team m	iembers U.Wal	sh, M. Deschens	os, L. Borochars
<ol> <li>Dec</li> <li>Stay</li> <li>Cur</li> <li>Med</li> <li>Side</li> <li>Par</li> <li>Rai</li> <li>Ant</li> <li>Light</li> <li>Util</li> <li>Dec</li> </ol>	dian ewalks apet ling i Missile Fence tins hting Standards	45 Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z Z	1. Bearin 2. Stringe 3. Diaphr 4. Girder 5. Floor I 6. Trusse 7. Rivets 8. Welds 9. Collisi 10. Load 11. Memb 12. Load 13. Paint- 14. Year I 15. Under	ragms s or Beams Beams es or Bolts con Damage Deflection per Alignmen Vibration	ft	5 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	item 60 SUBSTRUCTURE  1. Abutments a-Wings b-Backw c-Bridge d-Breast e-Footing f-Piles g-Erosio h-Settler 2. Piers or Bents a-Caps b-Colum c-Web d-Footin e-Piles f-Scour g-Settler 3. Collision Dama 4. Hydraulic-Ader	rall N Seats N Seats N T gs N ment N T T T T T T T T T T T T T T T T T T T
	ended Posting	Н 3 382	Single		Overhe	yes	(attached to bridge) no	
From Rati		at bridge	advar N	nce	2. Bolt 3. Con		2	
LEGIBILI	TY				Item93	Bb U/W I	nspection Date:	
2. emb	onnel scour cankment erosion der system r dikes & jetties	3 5. rip i 3 6. effe	rap or slope pectiveness	paving N	1. b 2. tr 3. a	raffic Saridge raili ansitions oproach ( uardrail to	guardrail	3 3 3

PROJECT: Franklin Fall: Dam

NAME: Upper M: 11 Brook Bridge

LOCATION: Hill NH

## BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	y 25
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	ye5
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructure with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway openings.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	yes yes yes yes
3. Are any characteristics of an aggressive stream or waterway present?	yes
<ul> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	yes yes des yes
4. Is the bridge located on a stream reach with any adverse flow characteristics?	૧૦
<ul> <li>a. Crossing near stream confluence.</li> <li>b. Crossing of tributary stream near confluence with larger streams.</li> <li>c. Crossing on sharp bend in stream.</li> <li>d. Location on alluvial fan.</li> <li>e. Other.</li> </ul>	no no no
5. Other comments or observations.	yes
Erosion beneath northeast embankment causing slump.	

# FRANKLIN FALLS DAM KNOX BROOK BRIDGE FISCAL YEAR 1993 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION:

14 July 93

DATE OF PREVIOUS INSPECTIONS:

Routine Inspection, 14 May 91 Inventory Inspection, April 85

#### RATING (T = TONS)

Type	Inventory	Operating	Comments
Н .	20 T Esti	mated	The bridge was reconstructed in 1992. Load rating calculations are forthcoming.

### **EVALUATION** (See attached "Structures Inspection Field Report")

A. Superstructure
-Above Deck

-Overall condition is very good.

-The bridge was rebuilt in 1991. The stone abutments were capped with new concrete bridge seats. The superstructure is constructed of prestressed concrete planks.

-Both north and south approaches are in fair condition. The gravel roadway is slightly rutted as it transitions to the bridge deck.

-The southeast stone wingwall is capped with three W12 steel beams, presumably salvaged from the old superstructure.

-The joint at the interface between the south west stone wingwall and the new concrete abutment is wide and allows gravel to wash down off of the road.

-There are no approach guard rails or bridge railings.

-Joints between the deck and both abutment backwalls are improperly sealed with concrete. This is causing some cracking and spalling at the joint.

-All of the wingwalls are in good condition, with only moderate growth of vegetation between the stones.

В.	Superst	ructure
	-Below	Deck

-Overall condition is very good.

-There is a foam backer rod protruding from between the two eastern precast planks.

-Underside of deck is in good condition.

#### C. Substructure

-Overall condition is good.

-The stone abutments are in good condition.

-There is a beaver dam constructed against the upstream (east) wingwalls.

-Clear water is flowing out from between the stones of both abutments. The water is flowing from behind the beaver dam. The water does not appear to be carrying soils out from behind the abutments.

-Slight scour is present under the north abutment.

D. Channel

- -The channel under the bridge is in fair condition, with only slight scouring.
- E. Overall Numerical Condition Rating

Inventory 1985: 7
Routine 1991: 4
Routine 1993: 9

#### RECOMMENDATIONS

#### Status of Previous Recommendations

- Post the load limit at both approaches.
- 2. Remove existing deck and girders, and recap both abutments with new concrete bridge seats. Install a new prestressed concrete plank bridge deck with parapets.
- Install guard rails on both approaches.

Item No. 2 has been completed. Items 1 and 3 have not.

#### Revised Recommendations

Post the load limit at both approaches.

### STRUCTURES INSPECTION FIELD REPORT

### **ROUTINE INSPECTION**

city/te San Bornt	ton 1/H	b	ridge dept. no.	8-struct	ure no. UED NH 33 10006	90-date inspected
2-dist. 104-highway		22-owner	27-yea		106-year rebuilt	11-milepoint
	Federal Aid	COE		920	1992	
43-structure type <i>Prestressed</i> , F	Precast Concr	ete Plank.	5	control en		
07-facility carried  **Reservoir** Account Accounts**	cess Road		team l	Joe.	Colucci	
06-features intersected  Knox Groot				members lalsh	M. Deschenes , L	l. Bravchaner
item 58	9	item 59 SUPERSTRU	ICTURE	9	item 60_ SUBSTRUCTURE	8
DECK		1. Bearing I		9	1. Abutments	
Wearing Surface	9	2. Stringers		N	a-Wings	7
2. Deck-Condition	9	3. Diaphrag		.N	b-Backw c-Bridge	
<ol><li>Stay in Place For</li></ol>			Beams Planks	9	d-Breast	
4. Curbs	9	5. Floor Bea		N	e-Footing	
5. Median	N	6. Trusses		N	f-Piles	N
6. Sidewalks	N	7. Rivets or	Bolts	N	g-Erosio	
7. Parapet	N	8. Welds		N	h-Settlen	nent 🖊
8. Railing	N	9. Collision	Damage	N	2. Piers or Bents a-Caps	W
9. Anti Missile Fenc	e N	10. Load Def	lection	9	b-Colum	n 📈
10. Drains	N	11. Member	Alignment	9	c-Web	N
11. Lighting Standard	is N	12. Load Vib	ration	N	d-Footing	
12. Utilities	N	13. Paint-Epo	эху	N	e-Piles	nent W
13. Deck Joints	7	14. Year Pair	nted	N	f-Scour	<i>N</i>
		15. Under Cl	earance ft	in	g-Settler 3. Collision Dama	nent 20
14. Approach Settlen	nent 🗀	Clearance	Signs y	es no		
Actual Posting	H 3 3S2	Single	Overh	1	(attached to bridge)	
, total in the same				yes	[X] no	
Recommended Posting			1. We	lds	N	
From Rating Book		<u></u> J	0.00		N	
SIGNS IN PLACE	at bridge	advance	2. Bol	ıs		
Y or N	W	W		ndition		
LEGIBILITY			Item9	3b U/W	Inspection Date: MA	
( TEM 61-channel as	nd channel protection	on 7	36-	Traffic Sa	fety features	
2. embankment ero 3. fender system 4. spur dikes & jetti	sion 7 6. effe 2 7. deb	ap or slope pavictiveness ris etation	7 2. tr 7 3. a	ridge raili ansitions pproach g uardrail t	guardrail 8	condition

#### SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	<u>/es</u> _
2. Is streambed erodible? If so, does the structure have any vulnerable design features?	<u>/es_</u> _
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructures with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway opening.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	Stone Abutment
<ul> <li>3. Are any characteristics of an aggressive stream or waterway present?</li> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	N N 
4. Is bridge located on stream reach with any adverse flow characteristics?	
<ul> <li>a. Crossing near stream confluence.</li> <li>b. Crossing of tributary stream near confluence with larger streams.</li> <li>c. Crossing on sharp bend in stream.</li> <li>d. Location on alluvial fan.</li> <li>e. Other.</li> </ul>	

5. Other comments or observations.

## FRANKLIN FALLS DAM BLAKE BROOK BRIDGE, NEW HAMPTON, NH FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 14 July 93

<u>DATE OF PREVIOUS INSPECTIONS</u>: Inventory, April 85

Routine, 16 July 87 Routine, 30 May 89 Routine, 14 May 91

#### RATING (T=TONS)

Type	Inventory	Operating	Comments
H10	14.5T	19.4T	Load capacity has not changed since previous inspection.

#### EVALUATION (See attached "Structures Inspection Field Report")

A. Superstructure
Above Deck

The overall condition of the superstructure is good. (condition) 7 There is some minor rutting at each of the gravel approaches. The new timber deck is in very good condition. Sand is accumulating along the brush blocks on either side of the bridge and is preventing adequate drainage of the The 15 ton rating signs at bridge deck. each bridge approach have been vandalized with graffiti and are illegible. The guardrails are in good One post at the north end of condition. the west guardrail is loose. One bolt is missing on the west guardrail at the third support from the south approach. Vegetation is encroaching upon each approach.

B. Superstructure Under Deck

The overall condition of the substructure is good (condition 7). There is minor to moderate rusting of all structural steel. The existing paint system is in poor condition. There is minor debris build-up along the flanges of the steel.

C. Substructure

The overall condition is good (condition 7). The wingwalls and abutments are in good condition with only very minor cracking and efflorescence noted. There

are no signs of settlement or scour. One weep hole in the south abutment is plugged.

#### RECOMMENDATIONS

#### Status of Previous Recommendations

1. Clean debris from deck and bottom flanges Not Done of the girders. Fill, grade and compact rutted areas of the approaches. Remove obstruction from south abutment weep hole.

2. Clean and repaint all structural steel Not done

3. Replace the guardrail support along the Not Done north end of the west guardrail.

Revised Recommendations

Implement above recommendations

Total Updated Estimated Cost \$15,000

### STRUCTURES INSPECTION FIELD REPORT

### ROUTINE INSPECTION

		_	bridge dept. no.	8-structur		90-date inspected
H	H BRISTOL	FRANKLIN FALL	& COE		DNH381 0001	14 July 93
dist.	104-highway system	22-owner	27-yea		106-year rebuilt	11-milepoint
3-structur	re type		quality	control engi	neer	
_	LED BEAM B	ZIDGE UN TIA	1BBR OSCK	NICK	- FURBES	
7-facility				leader		
				SOF C	مدردر ا	
6-feature	s intersected		team	members		
	BLAKE BROOK		N.	DESCHEN	es M. Nash	BORKHADER
item 58	8	7 item 59		7	item 60	7
		SUPERS	TRUCTURE	الت	SUBSTRUCTURE	
DECK			ng Devices	AL	<ol> <li>Abutments</li> </ol>	
1W	earing Surface			NY	a-Wings	7
2. De	eck-Condition	8 2. String		NA	b-Backw	
3. 5	tay in Place Forms	3. Diaph	-		c-Bridge	
		4. Girde	rs or Beams		d-Breast	
4. C		5. Floor	Beams	NA	e-Footing	
5. M	ledian	6. Truss	ses	20	f-Piles	AA
6. S	idewalks		s or Bolts	NA	g-Erosio	
7. P.	arapet	JA 8. Weld	S	اهن	h-Settler	
	ailing	9. Collis	sion Damage	ALA	2. Piers or Bents	
			Deflection	ALI	a-Caps	n NA
9. A	nti Missile Fence	10. 2000		Ja	b-Colum c-Web	77
10. D	rains		ber Alignment	NA.		(TSA)
11. L	ighting Standards		Vibration	1-13	d-Footin e-Piles	
12 11	Itilities	JA 13. Paint	:-Epoxy	<u>0</u>	f-Scour	EVA)
		NA 14. Year	Painted	8586	g-Settle	20
	eck Joints		er Clearance	ft in	3. Collision Dama	(35)
14. A	approach Settlement		nce Signs	yes 🛺 no	4. Hydraulic-Ade	
	•		·	- (	4. Hydradic-Ade	quacy
			Ove	head Signs	(attached to bridge)	
Actual F	Posting H 3	3S2 Singl	e	yes	X no	
	NA D	A NA 15				
Recomm	mended Posting		- 4 14	/oldo	464	
	ating Book	K	. vv	/elds		
			2. B	olts	NA	
SIGNS	IN PLACE at bridg	e adva		**.*	NA	
Y or N	Y		3. C	ondition		
	٦	_				
LEGIBI	LITY 5		Item	193b U/W I	nspection Date: ບານຕໍ	5
ITEN	/ 61-channel and channe	protection	36	S-Traffic Sa	fety features	
					36	condition
	annel scour	5. rip rap or slope		bridge raili	ng 🗓	
	nbankment erosion	6. effectiveness		transitions	ruardrail (	4
	ilder system	7. debris		approach g guardrail te		7
4. sc	our dikes & jetties 🕒	8. vegetation	LL   4.	guardraii te	ziiiiilai 🚨	

PROJECT: FRANKLIN FALLS DAY NAME: BLAKE BROKE BRIDGE LOCATION: HILL, NEW HAMBHIRE

## BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	NO
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	Yes
a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other.	Yes No No No Yes
3. Are any characteristics of an aggressive stream or waterway present?	N
<ul> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	70 70 70 70
4. Is the bridge located on a stream reach with any adverse flow characteristics?	40
<ul> <li>a. Crossing near stream confluence.</li> <li>b. Crossing of tributary stream near confluence with larger streams.</li> <li>c. Crossing on sharp bend in stream.</li> <li>d. Location on alluvial fan.</li> <li>e. Other.</li> </ul>	20 20 20 20
5. Other comments or observations.	NONE

## FRANKLIN FALLS DAM OLD RT 3A BRIDGE, HILL/ BRISTOL, NH FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 14 July 93

DATE OF PREVIOUS INSPECTIONS: Inventory, June 84

Routine, 16 July 87 Routine, 31 May 89 Routine, 14 May 91

#### RATING (T=TONS)

Type	Inventory	Operating	Comments
Н15	11.3T	16.1T	It is recommended that the bridge be limited to 5 tons

#### EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway and Railings: The overall condition is fair (rating

5). The bituminous surface coating is in poor condition with various cracking along the deck. There is vegetation growth and a buildup of debris along both gutters. There are no guardrails at either approach. A safety gate at the north approach is no longer useable. The north approach is rutted with two large potholes approximately 15 feet before the bridge. The south approach is in good condition.

B. Fascias & Curbs:

The parapets on the bridge are in fair condition. There is extensive spalling along the parapet walls. The faces of the walls are covered with graffiti. The anchor bolts supporting the access gate have pulled out of the parapet, and the gate is no longer usable.

C. Underside of Deck:

The overall condition is good. The northern end of the deck diaphragm measured 13" from the breast wall to the back face of the diaphragm (11" 1991 Routine inspection). This would prove that the abutments have moved since the previous inspection. There is some hairline cracking along the concrete tee beams in both transverse and longitudinal directions. The longitudinal cracking

apparent at the approximate center of the east and center tee beams may be due to insufficient cover. Some transverse cracking noted in previous inspections may be caused by excessive shear stresses. The west beam is in good condition.

#### D. Wingwalls/Abutments:

The overall condition of the wingwalls and abutments is poor (condition 3). in the north abutment appears to have worsened. The crack now measures 5" at top and 1 1/2" at the bottom. The footing is covered in this area but is suspected to be cracked as well. The west wingwall has dropped 1" lower than the breastwall. north abutment is rotated approximately 3 to 5 degrees south and is suspected to have moved since the last inspection. The south abutment has a similar crack at the east side of the breastwall. This crack measures 2 1/4" at the top and 3/4" at the bottom and continues through the footing. The east wingwall has rotated almost 1 3/4" east from This abutment has the top of the abutment. rotated 3 to 5 degrees north. It is difficult to asses whether this wall has undergone any additional movement. abutments appear to have rotated almost 3 inches since the 1984 in-depth inspection and almost 1 foot since construction.

#### E. Channel:

The overall channel rating is 5. The previous inspection stated that the hydraulic adequacy of the bridge opening is poor. A scour analysis has been performed and is included in the 1992 bridge inspection report appendix A. The area of scour along the south abutment did not appear to be as deep as stated in the 1991 routine inspection.

CONDITION RATING:	1984	1987	1989	1990	1991	1993
	A/E	NED	NED	NH DOT	NED	NED
Deck	6	6	6	7	6	6
Superstructure	7	6	5	7	5	5
Substructure	6	5	5	4	4	4
Channel	N/R	7	. 7	5	5	5
Approaches	6	6	5	4	6	5

#### Status of Previous Recommendations

1. Post Bridge at 5T gross load to restrict traffic to a pickup truck or less.

Not Done

2. Place guardrail and terminal at the north-west approach

Not Done

3. Place a timber crib to arrest the erosion pass flow adjacent to the northwest wingwall.

Not Done

4. Instrument the cracks, abutments and deck with devices to measure movement more accurately Not Done

#### Revised Recommendation

Due to the severity of the failure and the apparent movement of the bridge in recent years, total replacement is considered the only practical solution to the problem of the abutment failure. Replacement of the bridge will also allow for an increased load carrying capacity for the bridge. It presently functions as emergency access to the reservoir.

Estimated Cost

\$175,000

#### Interim Recommendations

1. The bridge should be posted for a 5 ton weight rating and a 10 mph speed limit in order to limit traffic to a pickup truck or light duty dump truck.

Estimated Cost

\$500

- 2. Heavy trucks such as fire apparatus emergency vehicles and light excavation equipment, (backhoe or lighter) should be limited, unloaded, driven slowly, and carefully supervised while travelling over the bridge.
- 3. The street gate presently lying by the bridge should be repaired so that it can be locked. Provisions should also be made so that it will allow pedestrians to cross the bridge easily.

Estimated Cost

\$1000

4. Set reference points and markers in order to monitor the movements of the bridge. Inspect and record movements of the bridge twice per year and include the results of these inspections in the annual bridge inspection report.

Estimated Costs

\$2000 initial survey \$2000 per year

### STRUCTURES INSPECTION FIELD REPORT

### ROUTINE INSPECTION

bridge dept						
HILL -BRISTOL FRANKL	IN FALLS	COE			N EDNH 3310008	
2-dist. 104-highway system	22-owner		27-year t		106-year rebuilt	11-milepoint
X NON-FED	COE		192		NIA	2. Znei Soi BRISTOL
40 Stradition type				ontrol eng		
1312111	T- BCAM				Forbes	
07-facility carried	en all manual error of	Ţ	team lea		a marked finale l	
REL AREA ALCESS (SC	O RT SM	<i>l</i>	team m		Sand installed brooking	
06-features intersected				SC HEN	GE /MILALALL	But was Not
South RIVER			I NO DE	SC HEN	AS / IN. ISSANT IS	. F3.14 a. L
item 58 DECK		RUCTURE		5	SUBSTRUCTURE  1. Abutments	. 4
1. Wearing Surface		g Devices		NA	a-Wings	4
2. Deck-Condition	2. Stringe			7	b-Backwa	
3. Stay in Place Forms	3. Diaphi				c-Bridge S	
4. Curbs	4. Girder	s or Beams		6	d-Breastw	<del></del>
. 50.25	5. Floor I	Beams		NA	e-Footings	ها ا
5. Median	6. Trusse	es		NA	f-Piles	
6. Sidewalks	7. Rivets	or Bolts		NA	g-Erosion h-Settlem	
7. Parapet 5	8. Welds	;		NA	2. Piers or Bents	ent Lo
8. Railing	9. Collisi	on Damage		WA	a-Caps	AC
9. Anti Missile Fence	10. Load	Deflection		di	b-Column	
10. Drains	11. Memb	er Alignmen	it	. 5	c-Web	N
	12. Load	Vibration		NA	d-Footing	
VI. Eighting Standards	13. Paint-	Ероху		NA	e-Piles	NA
12. Otilities	14. Year	•		NA	f-Scour	ALA
13. Deck Joints		r Clearance	NA ft.	in	g-Settlem	
14. Approach Settlement 5		ce Signs		s 🛵 no	3. Collision Damag	
	Clearan	ce Signs	ye	3 [DAT 1.0	4. Hydraulic-Adeq	uacy
	Cimarla		Overhe	ad Signs	(attached to bridge)	
Actual Posting H 3 3S2	Single			yes	X no	
	10					
Recommended Posting	5		1. Weld	ds	AN	
From Rating Book			O Dalte		46	
SIGNS IN PLACE at bridge	adva	nce	2. Bolts	•		
Y or N	N	_	3. Con	dition	A	
	-					
LEGIBILITY	يم له	A)	Item93	b U/W I	nspection Date: NoN	6
ITEM 61-channel and channel protect	ion 5		36-T	raffic Sa	fety features	condition
T. F		aving 5	4 4-	idas roili	36	condition
	rap or slope p ectiveness	baving 5	1. Dr	idge raili ansitions		7
3. fender system AA 7. debris			3. approach guardrail			
	getation	8		uardrail t		NH

PROJECT: FRANKLIN FALLS

NAME: SLITH RIJBE

LOCATION: CXD ET 3A

## BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	Y65_
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	Yes_
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructure with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway openings.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	Yes Yes Yes Nove
3. Are any characteristics of an aggressive stream or waterway present?	Yes_
<ul> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	465 465 465 UNKINIJ
4. Is the bridge located on a stream reach with any adverse flow characteristics?	
<ul> <li>a. Crossing near stream confluence.</li> <li>b. Crossing of tributary stream near confluence with larger streams.</li> <li>c. Crossing on sharp bend in stream.</li> <li>d. Location on alluvial fan.</li> <li>e. Other.</li> </ul>	NO NO NO NO NO NO NO NO NO NO NO NO NO N
5. Other comments or observations.	NINE

## BIRCH HILL DAM MIDDLE ROAD BRIDGE, WINCHENDON, MA FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 25 June 93

DATE OF PREVIOUS INSPECTIONS: Inventory, Routine, September 87

Routine, 18 July 89 Routine, 11 July 91

#### RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	22T	35T	Load Capacity posted
3	32T	49T	15T (to limit heavy
3S2	<b>48T</b>	74 <b>T</b>	truck traffic in
3-3	61T	94T	recreation area)

#### EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway and Railings: Overall rating is 7. Access is limited

by locked gates which prohibit unauthorized access to the bridge. The buildup of pine needles continues to be

a problem since there is restricted traffic over the bridge. The joint sealant at both ends of the prestressed concrete plank has deteriorated. The joint sealant has unbonded and the joint is filled with debris. The bituminous surface of the deck is uneven which may cause some minor ponding. The railings

are in good condition. Approach guardrails are not present and are not warranted due to the restricted access

to the bridge. One bolt on the guardrail is missing as noted in the

previous inspection.

B. Fascias & Curbs:

Overall rating is 7. The hairline cracks reported in previous inspections and the inventory inspection have not appeared to have worsened. Some efflorescence from the cracks was noted in this inspection.

C. Underside of Deck:

Overall rating is 7. Minor seepage and efflorescence was noticed between precast concrete planks near the bearing pads. This seepage could be due to water passing through the failed joint sealer and following the joints in the planks. Alignment of the planks is good with no evidence of differential

movement or deflection.

D. Wingwalls/Abutments: Overall rating is 8. The new cast concrete abutments are in good condition with no signs of distress or settlement. No erosion was noted.

E. Channel:

Overall rating is 7. Debris was getting caught under the bridge causing a slight restriction in flow under the bridge.

CONDITION RATINGS

Inventory 1984: Routine 1987: Routine 1989: Routine 1991: 7 Routine 1993:

#### **RECOMMENDATIONS:**

#### Status of Previous Recommendations

1. Schedule annual maintenance to include spot painting of posts, replacement of missing hardware, sweeping deck, and cleaning debris from bridge seat.

Some maintenance done. Muse up annually. done. Must be kept

2. Reapply sealant at expansion joints Not Done

#### Revised Recommendations

- 1. Clean expansion joints and reapply sealer to both joints. butyl based or polyurethane based sealant (Sikaflex-15LM or equivalent). Estimated cost \$500.
- 2. Include in annual maintenance, cleaning the debris beneath the bridge from the brook.

## ROUTINE INSPECTION

		bridge dept	no.	8-structur	e no.	90-date inspected
BIRCH HILL DAM WIN	CHENDON LIA	COE			MA 2510013	6124191
2-dist. 104-highway system	22-owner	, 000	27-year b		106-year rebuilt	11-milepoint
III NON-FED	CDE		X		1979	. 1
43-structure type				ntrol engi		
PRESTRESSED CONCETE	JLAG (COI)			c Foes	365	
07-facility carried		. 1	team lea		•	
MIDDLE PD (REC DEED	. MILEST, PUBLIC RE	STRETED)			COLUCCI	
06-features intersected			team m		/	
TRIEST BROOK			M, D	esc.Ha	des / F. Full 6	
item 58	7 <u>item 59</u>			8	item 60	8
DECK		TRUCTURE			SUBSTRUCTURE	<u>:</u>
	1. Bearin	g Devices		2	Abutments	8
Wearing Surface	2. String	ers		N	a-Wings b-Backw	
2. Deck-Condition			*	46	c-Bridge	
3. Stay in Place Forms	A Girdo	s or Beams		B	d-Breast	[ <del>2</del> ]
4. Curbs	5 Floor			44	e-Footing	[27]
5. Median	6. Truss			NA	f-Piles	<b>14</b>
6. Sidewalks	NA	or Bolts		44	g-Erosio	
7. Parapet	8. Welds			ALI	h-Settler	
		ion Damage		AL	2. Piers or Bents	
8. Railing	0.00	Deflection		×	a-Caps	
9. Anti Missile Fence				8	b-Colum	in NA
10. Drains		oer Alignmen	ι	44	c-Web d-Footin	
11. Lighting Standards	JA 12. Load			AM	e-Piles	44
12. Utilities	NA 13. Paint	•		NA	f-Scour	
13. Deck Joints	<b>6</b> 14. Year		16	141.	g-Settle	ment Nr
14. Approach Settlement	15. Unde	r Clearance -	<u>իր</u> քե -	in	3. Collision Dama	age 🔼
. Approach cetternent	Clearan	ce Signs	ye:	s no	4. Hydraulic-Ade	quacy
			Overhe	ad Signs (	(attached to bridge)	
Actual Posting H	3 3S2 Single			yes	no	
	151					
Recommended Posting	22 08	1	1. Weld	is	44	
From Rating Book 22	32 48				44	
	a d		2. Bolts	5		
SIGNS IN PLACE at bridg	ge adva <b>\b</b>	_	3. Cond	dition	NE	
	<u> </u>					
LEGIBILITY 8			Item93	h H/W Ir	nspection Date: NOW	E
			11611130	- O/ VV 11	.opoolion buto.	
TEM 61-channel and channel	el protection 8		36-T	raffic Saf	ety features	
					36	
r. channel scour	<u>පි</u> 5. rip rap or slope p පි 6. effectiveness	paving N	1. br	idge railir ansitions	ng 1	8
	6. effectiveness 7. debris	Daving PA	2. tra	ansitions oproach g	uardrail N	i Ei
4. spur dikes & jetties	8. vegetation	ACI	4. gt	ardrail te		

PROJECT: BIRCH HILL DAM

NAME: MIDDLE ROAD BRIDGE

LOCATION: WINCHENDON, MA.

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	100
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	yes_
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructure with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway openings.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	yes yes no yes yes
3. Are any characteristics of an aggressive stream or waterway present?	No
<ul> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	
4. Is the bridge located on a stream reach with any adverse flow characteristics?	_ No
a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other.	7545E
5. Other comments or observations.	No-5

# BIRCH HILL DAM NEW BOSTON ROAD BRIDGE, WINCHENDON, MA FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 24 June 93

<u>DATE OF PREVIOUS INSPECTIONS</u>: Inventory, Routine, 24 September 84 September 87

Routine, 18 July 89 Routine, 11 July 91

#### RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	19T	33 <b>T</b>	Load Capacity posted
3	24T	40T	15T (to limit heavy
3S2	37 <b>T</b>	62T	truck traffic in
3-3	46T	77 <b>T</b>	recreation area)

#### EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway and Railings: Overall rating is 8. A contract to repair the deck, approaches and railings

was completed in 1992. The deck was

overlaid with a new 2" bituminous paving course. The approaches were also

repaired. New guardrails at each approach were installed. New

elastomeric joint sealer was installed.

Some minor settling and erosion was noticed around some of the new guardrail posts and gabions. There are slight depressions in the approach pavements at the expansion joints which could collect

water or create a rough transition onto the bridge deck.

B. Curbs, Fascias:

Overall condition is 7. The concrete in the curbs and fascias is good. The minor hairline cracking in the curbs has not appeared to have worsened since the previous inspection.

C. Underside of Deck:

Overall condition is 8. The concrete in the precast planks is good. Some minor seepage and efflorescence was noticed on the underside of the deck along the longitudinal joints and around the bearings. The efflorescence may have been from previous seepage through the expansion joints prior to replacement of the joint sealer. Alignment is good. The elastomeric bearing pads are also in

good condition.

Overall condition is 8. The concrete D. Wingwalls/Abutments:

cap over the original stone

foundation is in good condition.

wingwalls have been protected by

installing new gabions which have also helped prevent erosion from runoff from the deck. The erosion on the southwest

bank has been repaired with stone protection and is functioning well.

The overall condition is 8. The E. Channel

streambed was clear of debris and shows

no sign of scour.

Inventory, 1984 CONDITION RATING

Routine, 1987 7 Routine, 1989 Routine, 1991 7 Routine, 1993

#### RECOMMENDATIONS:

#### Status of Previous Recommendations

1. Install "Narrow Bridge" signs. Not done

2. Install 30'± gabions. Completed 1992

3. Install 75'+ guardrail along south- Completed 1992 west approach. Install 45'+ quardrail other approaches.

4. Extend transition slabs. Replace Completed 1992 joint filler.

5. Schedule maintenance including Ongoing maintenance cleaning sand off bridge, debris off bridge seat, and cut back encroaching vegetation.

#### Revised Recommendations

Patch settling and eroding areas around new railing posts. Estimated cost \$500.

## **ROUTINE INSPECTION**

City, 10 mm	bridge	dept. no.	8-structur	e no.	90-date inspected
WINCHESDON, MA BIRCH HIL			CEPN	EDMA2510014	6/24/93
2-dist. 104-highway system	22-owner	27-year	The state of the s	106-year rebuilt	11-milepoint
III NON-FED	COE	190		1976	. 5
43-structure type		1	ontrol engi		
PRESTRESSED CON. SL	AB (501)		H FOR	1365	
07-facility carried		team le			
NEW BOSTON PRINT (P	EC NUESS)		EPH ( nembers	COLUCCI	
06-features intersected				ENES / F. FUN	
MILLER PIVER		M.,	DL-SC HE	ANES I F. TON	
item 58	item 59		8	item 60	8
DECK	SUPERSTRUCTU	JRE		SUBSTRUCTURE	
	1. Bearing Devic	es	8	1. Abutments	8
	2. Stringers		424	a-Wings	CAD
E. Deak Condition	3. Diaphragms		2	b-Backwa	
3. Stay in Place Forms	4. Girders or Bea	ams	8	c-Bridge d-Breasty	
4. Curbs	5. Floor Beams		44	e-Footing	1700
5. Median	6. Trusses		44	f-Piles	ACA
6. Sidewalks	7. Rivets or Bolts	s	NA	g-Erosion	
7. Parapet	8. Welds		24	h-Settlen	nent B
8. Railing	9. Collision Dam	age	44	2. Piers or Bents	LIA.
NA.	10. Load Deflection	_	X	a-Caps	AU AZ
5. 7 III.	11. Member Align		8	b-Columi c-Web	44
- 1	12. Load Vibratio		X	d-Footing	
Tr. Eighting Ottahodros		11	44	e-Piles	r\A
12. Utilities	<ol> <li>Paint-Epoxy</li> <li>Year Painted</li> </ol>		-3A	f-Scour	PA
13. Deck Joints		114 4	in	g-Settler	
14. Approach Settlement	15. Under Cleara			3. Collision Dama	
,	Clearance Signs	s ye	s NA no	4. Hydraulic-Adec	luacy 🖺
		Overhe	ead Signs (	attached to bridge)	
Actual Posting H 3 3S2	Single		yes	⊤ no	
	15				
Recommended Posting From Rating Book		1. Wel	ds	44	
From Rating Book		0 B-#	•	NA	
SIGNS IN PLACE at bridge	advance	2. Bolt	5		
Y or N	N	3. Con	dition	NA	
LEGIBILITY		Item93	Bb U/W In	spection Date:	E
		1			
ITEM 61-channel and channel protection	n 8	36-T	raffic Safe	ety features	
Shannel agour A F sin s	an or clone paving	NA 4 L	ridao railia	36	condition ঙ
	ap or slope paving ctiveness		ridge railin ansitions	ig ,	8
3. fender system 7. deb			oproach g	uardrail <u>I</u>	8
4. spur dikes & jetties NA 8. vege	atation	1 4 m	uardrail te	rminal	1 18

PROJECT: BIRCH HILL DAM

NAME: NEW BOSTEN BODD

LOCATION: WINCHESDEN, MA

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	yes
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	yes_
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructure with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway openings.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	yes No No Yes
3. Are any characteristics of an aggressive stream or waterway present?	<u>Nes</u>
<ul> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	20 20 20 20 20 20 20 20
4. Is the bridge located on a stream reach with any adverse flow characteristics?	ye-s
<ul> <li>a. Crossing near stream confluence.</li> <li>b. Crossing of tributary stream near confluence with larger streams.</li> <li>c. Crossing on sharp bend in stream.</li> <li>d. Location on alluvial fan.</li> <li>e. Other.</li> </ul>	No Yes Yes
5. Other comments or observations.	NONE

#### BIRCH HILL DAM BURGESS ROAD BRIDGE, WINCHENDON, MA FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 24 June 93

DATE OF PREVIOUS INSPECTIONS: Inventory, 24 September 84 Routine, September 87 18 July 89 Routine,

21 September 90 Routine,

#### RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	<b>30T</b>	47T	Load Capacity posted
3	43T	66 <b>T</b>	15T (to limit heavy
3S2	66T	101T	truck traffic in
3-3	84T	128T	recreation area)

#### **EVALUATION** (See attached "Structures Inspection Field Report")

A. Roadway and Railings: Overall rating is 7. There are several depressions and ruts in the deck and approach slab pavements. There is moderate vegetation growth and pine needles along both curbs. Vegetation at the southwest corner of the bridge has not been removed and is encroaching into the roadway as mentioned in previous inspections. The pavement at the expansion joints along the west approach sinks below the concrete edges Guardrails should be installed at each corner of the approaches as noted in previous inspections in order to ensure adequate safety for approaching vehicles.

B. Fascias and Curbs:

Overall condition is 7. The concrete in the curbs and fascias is in good condition. The hairline cracks in the curbs show some efflorescence and do not seem to have deteriorated since the last inspection. The approach curb at the south west corner of the bridge is cracked along the top which may eventually propagate into a spall.

C. Underside of Deck:

Overall condition is 7. The concrete in the precast planks is in good condition. The spall mentioned in the 1991 report could not be located, however the general area should be continually inspected in order to determine if there may be any deficiency in the concrete planks. There has been no change in the condition of the one inch differential between the precast concrete planks. Some seepage and efflorescence was noticed between the concrete planks near the bearing pads.

D. Wingwalls/Abutments:

Overall condition is 7. The concrete caps over the original rubble masonry are good. The elastomeric bearing pads are also in good condition. The abutments show no signs of settlement, deterioration or scour.

E. Channel:

The overall condition of the channel is 7. The brook was flowing smoothly, however, debris was building up under the bridge, creating a slight obstruction to flow.

CONDITION RATING

Inventory, 1984 8
Routine, 1987 8
Routine, 1989 7
Routine, 1991 7
Routine, 1993 7

#### RECOMMENDATIONS

#### Status of Previous Recommendations

1.	Install	"Narrow	Bridge"	warning	Not	done
	signs.					

Install guardrail at approaches Not done

 Extend transition slabs, install Not done drainage, and seal expansion joints.

#### Revised Recommendations

Install 75' $\pm$  of guardrail at the approach at the southwest wingwall and remove encroaching vegetation. Install 45' $\pm$  of guardrail at each of the other three corners of the bridge. 3"x10" pressure treated rails with 8"x8" pressure treated posts are recommended. Replace the joint sealant in the expansion joints. Estimated cost \$7500.

## **ROUTINE INSPECTION**

c C			bridge dep	t. no.	8-structu	ıre no.	90-date inspected
WINEH ELDOW, MA B	RCH HIL	L DAM			CEPNE	ED MA 2510015	6/24/93
2-dist. 104-highway system		22-owner COE		27-year	built	106-year rebuilt	11-milepoint
43-structure type					ontrol eng	jineer	
	TE SLAC	(501)		^	JICK	FORBES	
07-facility carried				team le	ader		
BIRGESS RD.				T	COLL	1661	
06-features intersected					nembers	1 - 1 1 - 1 1	
PRIEST BROOK				M.D	55C HEN	les / F. FUNG	
item 58	7	item 59			8	item 60	8
DECK		SUPERST	RUCTURE			SUBSTRUCTURE	
Wearing Surface	7	1. Bearing	Devices		8	1. Abutments	
Deck-Condition	7	2. Stringe	rs		N	a-Wings b-Backwa	
	NA	3. Diaphr	agms		44	c-Bridge	
3. Stay in Place Forms	7	4. Girders	or Beams		8	d-Breastv	wall 8
4. Curbs	NA	5. Floor B	Beams		AL	e-Footing	
5. Median		6. Trusse	s		NA	f-Piles	21
6. Sidewalks	NA	7. Rivets	or Bolts		NA	g-Erosior	( <del></del>
7. Parapet	Ah	8. Welds			44	h-Settlem 2. Piers or Bents	ient 6
8. Railing	7	9. Collisio	n Damage		NA	a-Caps	NA
9. Anti Missile Fence	NA	10. Load D	eflection			b-Column	
10. Drains	AL	11. Membe	er Alignmen	t	1	c-Web	NA
11. Lighting Standards	NA	12. Load V	ibration			d-Footing	
12. Utilities	AL	13. Paint-E	Броху		44	e-Piles	AA HA
13. Deck Joints	6	14. Year P	ainted		44	f-Scour	NA NA
	6	15. Under	Clearance -	NA ft.	in	g-Settlen 3. Collision Dama	-
14. Approach Settlement		Clearanc	e Signs	ye	s NA no	4. Hydraulic-Adeq	
						, 55	
				Overhe	ad Signs	(attached to bridge)	
Actual Posting H	3 3\$2	Single			yes	on [4]	
		150					
Recommended Posting From Rating Book	43 66			1. Weld	ds	N	
FIOH Halling Book				2. Bolts		NA	
SIGNS IN PLACE at bri	dae	advan	ce	2. DOIIS	•		
Y or N	]	<b>P</b> .		3. Con	dition	Ah	
	7		-				•
LEGIBILITY B				Item93	b U/W Ir	nspection Date: None	•
'TEM 61-channel and chan	nel protectio	on G		36-T	raffic Saf	ety features	
The strained and offering		<u>8</u>		00-1	, and oar	36 <u>36</u>	condition
1. Grannel scour	8 5. rip r	ap or slope pa	aving DA		idge railir		7
<ol> <li>embankment erosion</li> <li>fender system</li> </ol>	8 6. effect  7. deb	ctiveness	4		ansitions proach g	u ardrail D	7
4. spur dikes & jetties	가요 8. vege		NA		iproach g iardrail te		

PROJECT: BIRCH HILL

NAME: BURGESS RD

LOCATION: WINCHENDY, ML

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	NO
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	Ycs
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructure with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway openings.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	yes yes yes
3. Are any characteristics of an aggressive stream or waterway present?	٥٥
<ul> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	No No Nes Nes
4. Is the bridge located on a stream reach with any adverse flow characteristics?	No
<ul> <li>a. Crossing near stream confluence.</li> <li>b. Crossing of tributary stream near confluence with larger streams.</li> <li>c. Crossing on sharp bend in stream.</li> <li>d. Location on alluvial fan.</li> <li>e. Other.</li> </ul>	No No No No
5. Other comments or observations.	none

# BIRCH HILL DAM OLD ROUTE 202 BRIDGE, WINCHENDON MA. FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 24 June 93

DATE OF PREVIOUS INSPECTIONS: Inventory, 24 May 84.

Routine, Sep 87. Routine, 29 Jul 89.

Routine, 21 Sep 90.

#### RATING (T = TONS)

Type	Inventory	Operating	Comments
H15	23T	35T	Load rating
3	34 <b>T</b>	53T	recalculated due
3S2	54 <b>T</b>	84T	to new deck
3-3	66T	103T	concrete overlay.

A. Roadway and Railings

Overall rating 7. The bridge west approach showed some depression but the overall transition to the concrete deck is smooth. The approach guardrail, bridge rails, concrete overlay and transition slabs are in good condition. The approach guardrail are far from the pavement but they are functional. Slight erosion is located at the southwest and northeast approach corner.

B. Fascias

Overall condition is 8. Both fascia and bridge deck are in good condition. No cracks or concrete spall were located. Bridge deck is also in excellent condition. The deck drainage and weep holes are clear.

C. Underside of Deck

Overall condition is 8. The beams and deck diaphragm do not have any sign of concrete spall. No cracks or water staining was noted. The underside of the deck is in good condition.

D. Wingwalls & Abutments

Overall condition 7. The wingwalls are in good condition. There are is erosion at the bottom of bituminous waterway behind southeast wingwall. Also minor spall on the southwest corner of abutment with moss growth was noted. There are no signs of scour at the foundation.

E. Channel

Overall condition 7. The streambed under the bridge is filled with vegetation and tree branches. The stream flow was moderate during inspection; however, no major signs of scour were noted.

CON	ידת	TTC	M	DA'	TING
CON	$\nu_{\perp}$	$_{1}$	/1N	TCD	TING

Inventor	6	
Routine,	1987:	6
Routine,	1989:	5
Routine,	1991	7
Routine,	1993	7

#### RECOMMENDATIONS:

Status of Previous Recommendations

 Modify approach guardrail at transitions to guide around brush blocks.

Not done

2. Make miscellaneous patch repairs to abutments and wingwalls at flaws which were missed in 1990 contract or below the existing water level (contractor limit of work).

Not done

#### Revised Recommendations

 Remove all the tree branches, debris and other vegetation near and under the bridge deck. ( Project Personnel )

## ROUTINE INSPECTION

d O	WINCHEN	DON, MA		bridge dep	t. no.	8-structur		90-date inspected 6/24/93
2-dist.	104-highway sy		22-owner COE		27-year t		106-year rebuilt	11-milepoint
43-structur		ER / MULTI .	-BEAM OR	GIRDER	quality co	ontrol engi	neer	
07-facility	carried O	LD ROUTE	202		team lea		T. Colucci	
06-feature	es intersected  WATE	ER BEAUER	R BROO	K	team m		ESCHENES /F	FUNG
2. D 3. S 4. C 5. M 6. S 7. P 8. R 9. A 10. D 11. L 12. U 13. E	learing Surface eck-Condition tay in Place Forms urbs ledian idewalks arapet tailing anti Missile Fence	8 8 - 8 - 8 - 8	1. Bearin 2. String 3. Diaphi 4. Girder 5. Floor 6. Truss 7. Rivets 8. Welds 9. Collisi 10. Load 11. Memb 12. Load 13. Paint- 14. Year 15. Unde	ragms rs or Beams Beams es s or Bolts sion Damage Deflection per Alignmer Vibration Epoxy		8 - 8 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	item 60 SUBSTRUCTURE  1. Abutments a-Wings b-Backwa c-Bridge d-Breastv e-Footing f-Piles g-Erosior h-Settler  2. Piers or Bents a-Caps b-Column c-Web d-Footing e-Piles f-Scour g-Settler  3. Collision Dama 4. Hydraulic-Adec	all Seats Seats wall Seats sea
	Posting mended Posting ating Book	H 3 3S2 46 69 72	Single	e ]	Overhe	yes	(attached to bridge)    No	
SIGNS Y or N	IN PLACE	at bridge	adva	nce	2. Bolt 3. Con			
LEGIB	ILITY	<u>Y</u>		] .	Item93	Bb U/W I	nspection Date:	
1. ch 2. er 3. fe	M 61-channel and nannel scour mbankment erosion nder system our dikes & jetties	5. rip 7 6. effe 7 7. del	rap or slope pectiveness	paving –	1. b	raffic Sai ridge raili ansitions pproach q uardrail te	guardrail 0	condition  8 7 8

PROJECT: BIRCH HILL DAM
NAME: OID ROUTE 202
LOCATION: WINCHENDON, MA

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	YES
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructure with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway openings.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	YES YES YES NO YES
3. Are any characteristics of an aggressive stream or waterway present?	YES
a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. Concrete At Waterline	NO NO YES NO YES
4. Is the bridge located on a stream reach with any adverse flow characteristics?	
a. Crossing near stream confluence.	No
b. Crossing of tributary stream near confluence with larger streams.	No
c. Crossing on sharp bend in stream.	NO
<ul><li>d. Location on alluvial fan.</li><li>e. Other.</li></ul>	YES
5. Other comments or observations.	NONE

# BIRCH HILL DAM GOODNOW ROAD BRIDGE, WINCHENDON, MA FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 25 June 93

DATE OF PREVIOUS INSPECTIONS: Inventory,
Routine,
Deck reinforcing inspection,
Routine,
Routine,
Routine,
Routine,
25 September 84
4 September 87
4 September 87
21 September 90

#### RATING (T=TONS)

Type Inventory Operating Comments
H15 13T 18T No change in ratings

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway and Railings: Overall condition is good, no repairs needed (condition 8). The bituminous concrete deck overlay and transition

slabs are in good condition. All deck drains are clear and functioning

properly. The approach alignments are only 16 feet wide and slightly skewed. The timber approach and bridge railings

are in good condition.

B. Fascias & Curbs: The fascias and curbs are in good

condition.

C. Underside of Deck: The overall condition of the underside of the concrete deck is good. One spall

was noted in the concrete deck at approximately the third point of the outside east beam. The spall is approximately 12" long, 4"wide, and 4" deep. This spall has been noted in previous inspections, has not continued to deteriorate, and is not a concern at

this time.

D. Wingwalls/Abutments: The condition of the abutments and

wingwalls is good (condition 7). There are only minor hairline cracks with efflorescence on the east face of the north abutment. All other concrete is in good condition. The gabion retaining walls are in good condition. Erosion

was again noted beneath the south

abutment, and should be repaired.

E. Channel:

Overall condition 7. There is an existing area of scour beneath the south

abutment. The flow beneath the bridge

was swift with little obstruction.

Some minor rubble is deposited under the

bridge.

CONDITION RATINGS:

Inventory, 1984: 7
Routine, 1987: 7
Routine, 1989: 6
Routine, 1991: 7
Routine, 1993: 7

#### Status of Previous Recommendations

Item

Current Status

 Post warning signs "Narrow Bridge" on both approaches. Not Done

2. Repair scour at abutments.

Not Done

#### Revised Recommendations

Repair scour at abutments.

Estimated cost \$5,000.

## **ROUTINE INSPECTION**

P. C.				bridge dep		8-structur	e no. DNA 251 0017	90-date inspected 6/24/93
2-dist.	104-highway sy	, いんよんつ vstem	22-owner	1 WE	27-year	built	106-year rebuilt	11-milepoint
111	NON-F	ED	COE			37	1991	0,5
43-structure	type				' '	ontrol engir		
		LRETE DECK	(302)			V FORF	3-5	
07-facility ca					team le			
06-features		REC MED A	uss)		T	SEPH C nembers	8LCC1	
							S/F FUNG	
17216	ST BROOM				1 12. 12	SCHUNG	3 / 1 / 1000	
item 58		8	item 59	rni ioti inc		8	item 60 SUBSTRUCTURE	7
DECK				RUCTURE		8	1. Abutments	
1. Wea	aring Surface	8		g Devices		8	a-Wings	7
2. Dec	ck-Condition	8	<ol> <li>Stringe</li> <li>Diaphi</li> </ol>			8	b-Backwa	
3. Stay	y in Place Forms			s or Beams		16	c-Bridge d-Breasty	
4. Cur	bs	8	5. Floor I			ALA	e-Footing	
5. Med	dian	44	6. Trusse			44	f-Piles	N
6. Side	ewalks	72	7. Rivets			NA	g-Erosior	
7. Par	ranet	Air	8. Welds			44	h-Settlen	nent 🙎
8. Rai		8		on Damage		とろ	2. Piers or Bents	
l'		44		Deflection		$\overline{\times}$	a-Caps	n 44
	i Missile Fence	8			•	8	b-Columi c-Web	n 24
10. Dra	iins			er Alignmen	ı	X	d-Footing	
11. Ligi	hting Standards	DV	12. Load			8	e-Piles	44
12. Util	ities	AL AL	13. Paint- 14. Year I		160-		f-Scour	ACA
13. Dec	ck Joints	8	1	r Clearance .	1990 NA "	in	g-Settler	
14. Apr	proach Settleme	nt &					3. Collision Dama	
			Clearand	ce Signs	<b>N</b> ► ye	s no	4. Hydraulic-Adeo	quacy 7
				7,410				
			6: 1		Overhe	ead Signs (	attached to bridge)	
Actual Pos	sting	H 3 3S2	Single	1		yes	no	
		NA NA WA	12.1					
Recomme From Rati	ended Posting		137	-	1. Wel	ds	40	
From Hair	ing book				2. Bolt	•	AG	
SIGNS IN	N PLACE	at bridge	advar	nce	2. DOIL	3	NA	
Y or N			V	1	3. Con	dition	lots	
				-				
LEGIBILI	ΙΥ				Item93	Bb U/W In	spection Date: Nobe	-
I <u>IE</u> M 6	ITEM 61-channel and channel protection 6 36-Traffic Safety features							
				•			36	condition වේ
	2. embankment erosion 5. rip rap or slope paving 2. embankment erosion 7 6. effectiveness				1. bi	idge railin	g <u>1</u>	
	ankment erosi ler system	on אל. ene		Daving NA NA S	2. transitions 3. approach guardrail 4. quardrail terminal			
	r dikes & jetties		etation	NA	4. gi	uardrail te		$\overline{S}$
	· · · · · · · · · · · · · · · · · · ·							

PROJECT: BIECH HILL DAN NAME: 600 ADD ED LOCATION: WINCHESON, HA

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	yes
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	Yes
<ul><li>a. Piers, abutments with spread footings or short pile foundations.</li></ul>	yes
b. Superstructure with simple spans or non- redundant support systems.	ycs
<ul><li>c. Inadequate waterway openings.</li><li>d. Designs which collect ice and debris.</li></ul>	U0
e. All water must pass through or over structure.	yes
f. Other.	2
3. Are any characteristics of an aggressive stream or	
waterway present?	yes
a. Active degradation or aggredation of streambed.	yes
b. Significant lateral movement or erosion of streambanks.	405
c. Steep slopes.	no_
d. High velocities.	yes
e. Any history of highway or bridge damage during	
past floods.	no
f. Other. Staz undermining, Suth Abuthent Should	yes
4. Is the bridge located on a stream reach with any adverse flow characteristics?	yes
adverse from Characteristics.	
a. Crossing near stream confluence.	00
b. Crossing of tributary stream near confluence	
with larger streams.	00
c. Crossing on sharp bend in stream.	yes Yes
d. Location on alluvial fan. e. Other.	المان
e. Other.	<u> </u>
5. Other comments or observations.	none

# WEST HILL DAM WEST HILL ROAD BRIDGE, UXBRIDGE, MA FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 8 September 93

DATE OF PREVIOUS INSPECTIONS: Inventory,

Routine,

23 August 89 30 July 91

#### RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	12T	24T	No change in ratings Ratings based on satisfactory past performance without signs of distress.

EVALUATION (See attached "Structures Inspection Field Report")

A. Deck, Roadway & Railings:

Overall condition is 7. The roadway over the bridge is in good condition. Slight vegetation buildup was noticed along the granite curbs. The pavement along the northeast, and southwest wingwalls is beginning to erode due to runoff from the road. Steel quardrail sections that were installed along the northwest approach in order to control erosion are performing satisfactorily. The railings along the bridge deck are in good condition, however they are also very light duty and do not comply with the current AASHTO standards. There is a poor transition between the approach guardrails and the bridge deck railings along the north approach. The cables for the north approach guardrails are There are no guardrails loose. along the south approach. speed bumps at either end of the bridge are effective in controlling the speed of traffic. The bridge which is 18 feet wide is narrow and is currently used for two way traffic and pedestrians.

#### B. Superstructure/ Substructure

The overall condition is good (condition 8). The stones seem to be well bonded and aligned. is no sign of distress of the superstructure. The mortar grout on the underside of the arch is delaminating and spalling. It appears that during the 1940 rehabilitation of the bridge, the underside of the arch was formed in order to contain the flow of grout which was pressure injected from above the arch into the joints in the stone. In this case the thin mortar layer does not provide any additional structural strength and therefore the delaminating mortar There is some is not a concern. moss and vegetation growing from the joints between the stones. Some of the joints should be cleaned and repointed. The superstructure is primarily integral with substructure. substructure is also in good condition. Due to the depth of the water, the footings were not examined for scour potential.

#### C. Channel:

It was difficult to evaluate the overall condition of the channel. There was very light flow through the bridge at the time of the inspection. No erosion was noticed in the channel. The upstream channel makes a sharp turn south and another turn west before reaching the bridge.

#### CONDITION RATING:

1989	8
1991	8
1993	7

#### **RECOMMENDATIONS:**

#### Status of Previous Recommendations

1. Perform mortar joint repairs. Remove vegetation and repoint the joints over the stone arch on both sides.

Not Done

2. Control erosion and stabilize the west embankment.

Not Done

3. Install new approach and bridge guardrails.

Not Done

4. Install a pedestrian walkway.

Not Done

#### Revised Recommendations

A contract has been prepared during FY 93 to perform the above recommendations. No work had yet been accomplished by the time of the inspection.

### **ROUTINE INSPECTION**

NEST HILL DAM UXPRIDGE, N	bridge dep		ure no.	90-date inspected 8 SEPT 93	
	2-owner	27-year built	106-year rebuilt	11-milepoint	
NON FED	COE	1880	1940	.5	
43-structure type  STONE SPAN - STONE ARE	2.1+	quality control end	gineer CB8S		
07-facility carried		team leader			
WEST HILL ROAD LEGG.	Access)		Likei		
06-features intersected		team members			
WEST PIJER		H. DE60	HENGS FFUN	6	
item 58 DECK	item 59 SUPERSTRUCTURE	8	item 60 SUBSTRUCTURE	$\mathcal{E}$	
1. Wearing Surface	1. Bearing Devices	NK	1. Abutments	হ	
T. Wearing curiace	2. Stringers	ALA	a-Wings b-Backw	8   40   Ila	
2. 2001 00.10.110.11	3. Diaphragms	AC	c-Bridge		
3. Stay in Place Forms	4. Girders or Beams	NA	d-Breast		
4. Curbs	5. Floor Beams	nv.	e-Footing		
5. Median	6. Trusses	130	f-Piles	NA	
6. Sidewalks	7. Rivets or Bolts	NA	g-Erosio		
7. Parapet	8. Welds	NA	h-Settler	nent B	
8. Railing	9. Collision Damage	عنه	2. Piers or Bents		
9. Anti Missile Fence	10. Load Deflection	NA	a-Caps b-Colum		
10. Drains	11. Member Alignmer	nt B	c-Web	ĘA.	
14	12. Load Vibration	ACI	d-Footing		
11. Eighting Standards	13. Paint-Epoxy	N	e-Piles	PA	
12. Othites	14. Year Painted	المرد	f-Scour	nent Na	
13. Deck Joints	15. Under Clearance	ft <u>*)</u> in	g-Settler		
14. Approach Settlement	Clearance Signs	yes no	3. Collision Dama		
	Oreal alloc Olgilo	yes	4. Hydraulic-Adeo	quacy 🎦	
		Overhead Signs	(attached to bridge)		
Actual Posting H 3 3S2	Single 12	yes	✓ no		
	12			•	
Recommended Posting From Rating Book	15	1. Welds	AU		
From hatting book		2. Bolts	ACI		
SIGNS IN PLACE at bridge	advance	Z. Boits			
Y or N	7	<ol><li>Condition</li></ol>	NA		
	ALI	2000			
LEGIBILITY	[-75]	Item93b U/W	Inspection Date: <u>ドゥ</u>	JE	
ITEM 61-channel and channel protection 5 36-Traffic Safety features					
2. embankment erosion 5 6. effect	tiveness 3	2. transitions	0	7	
3. fender system 12 7. debris		3. approach		5	
4. spur dikes & jetties 2. veget	iation [8	4. guardrail t	erminai <u>0</u>	LA	
X≐UNKNOWN	NA=NOT APPLIC	ABLE	JA=INACC	ESSIBLE	

PROJECT: WEST HILL DAY PRIORE LOCATION: UKPLICE, MA

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	yes
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	yes
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructure with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway openings.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	Yes Yes No Yes
3. Are any characteristics of an aggressive stream or waterway present?	_Yes
<ul> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	100 100 100 100 1936-1940 1936-1940
4. Is the bridge located on a stream reach with any adverse flow characteristics?	<u> 4es</u>
<ul> <li>a. Crossing near stream confluence.</li> <li>b. Crossing of tributary stream near confluence with larger streams.</li> <li>c. Crossing on sharp bend in stream.</li> <li>d. Location on alluvial fan.</li> <li>e. Other.</li> </ul>	NO Nes No
5. Other comments or observations.	NONE

# THOMASTON DAM LEADMINE BROOK ROAD BRIDGE, THOMASTON, CT FISCAL YEAR 1993 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION:

24 August 93

DATE OF PREVIOUS INSPECTIONS:

Routine Inspection, 16 June 91 Inventory Inspection, November 84

#### RATING (T = TONS)

Туре	Inventory	Operating	Comments
H15	11 <b>T</b>	16T	No change in
Type 3	45T	69T	ratings due to
Type 3S2	69T	106T	inspection findings.
Type 3-3	86T	132T	

### EVALUATION (See attached "Structures Inspection Field Report")

A. Superstructure
-Above Deck

- -Overall condition is very good.
- -There are a few small potholes in the east approach pavement.
- -All of the approach stone walls are in good condition, with only minor vegetation growth between the pavement and the base of the wall.
- -The concrete transition slab at the east approach is in good condition.
- -The expansion joint at the east approach is in good condition.
- -The pavement at the west approach has a few bumps at the transition onto the deck.
- -Bridge railings and posts are in good condition. There are some minor shrinkage cracks in the surface coats of the concrete posts.
- -There is sand and debris accumulating on the deck near the south curb.
- -There are a few patches of deterioration in the bituminous wearing surface.
- B. Superstructure -Below Deck
- -Overall condition is good.
  - -The structural steel has recently been painted (1990), and is in good condition.
  - -Underside of deck is in good condition, with only minor honeycombing. There is some minor efflorescence coming from several transverse hairline cracks beneath both curbs.

Substructure

- -Overall condition is good.
- -The stone abutments are in good condition, with no signs of movement or settlement.
- -All of the four stone wingwalls are in good condition, with no signs of movement.
- -The east abutment is slightly undermined by scour.

D. Channel

- -The channel is scouring beneath the bridge. The channel is approximately four feet deeper under the bridge than it is either upstream or downstream of the bridge.
- Overall Numerical E. Condition Rating

Inventory 1985: Routine 1991: 8 Routine 1993:

8

#### RECOMMENDATIONS

#### Status of Previous Recommendations

- Inspect both abutments for scour. 1.
- Repair scour erosion at the south corner of the east abutment. 2. None of this work has been done.

#### Revised Recommendations

- Complete the scour analysis of the east abutment. The west 1. abutment is founded on rock and it is unlikely that it is susceptible to scour.
- Post a 10 Ton load limit at the east approach. 2.

## **ROUTINE INSPECTION**

Thomaston CT	bridge dep		e no. DC <i>T0910003</i>	90-date inspected 8/24/93
2-dist. 104-highway system 8 Non Federal Aid	22-owner 70 Carps Of Eng.		106-year rebuilt	11-milepoint
43-structure type 304 Single Span Steel Wide Flange	e Ceaus Com Dock	quality control engin	Nich Forbe	5
07-facility carried Brock Road	1	team leader	Colucci	
06-features intersected Leadmine Brook		team members M. Walsh,	1. Deschenes, E	Torio
item 58  DECK  1. Wearing Surface  2. Deck-Condition  3. Stay in Place Forms  4. Curbs  5. Median  6. Sidewalks  7. Parapet  8. Railing  9. Anti Missile Fence  10. Drains  11. Lighting Standards  12. Utilities  13. Deck Joints	item 59 SUPERSTRUCTURE  1. Bearing Devices 2. Stringers 3. Diaphragms 4. Girders or Beams 5. Floor Beams 6. Trusses 7. Rivets or Bolts 8. Welds 9. Collision Damage 10. Load Deflection 11. Member Alignment 12. Load Vibration 13. Paint-Epoxy 14. Year Painted	8 8 8 8 8 8 8 8	item 60 SUBSTRUCTURE  1. Abutments a-Wings b-Backwa c-Bridge d-Breastv e-Footing f-Piles g-Erosior h-Settlem 2. Piers or Bents a-Caps b-Column c-Web d-Footing e-Piles f-Scour	Seats
13. Deck Joints  14. Approach Settlement	15. Under Clearance _ Clearance Signs	ft in no	g-Settlen 3. Collision Dama 4. Hydraulic-Adeq	nent Z ge W uacy 8
Actual Posting H 3 3S2	Single N	Overhead Signs (a	nttached to bridge)	
Recommended Posting From Rating Book	10	1. Welds		
SIGNS IN PLACE at bridge Y or N  LEGIBILITY	advance	2. Bolts 3. Condition  Item93b U/W Ins	spection Date:	· · · · · · · · · · · · · · · · · · ·
2. embankment erosion 3. fender system 4. spur dikes & jetties  7 6. effe	rap or slope paving 4/2 ectiveness 7	36-Traffic Safer  1. bridge railing 2. transitions 3. approach gu 4. guardrail teri	ardrail Wall	condition  8 8 6 -

#### SCOUR CHECKLIST

1. Is the bridge <u>currently</u> experiencing, or does it have a history of, scour activity?	_ <u>Yes</u> _
2. Is streambed erodible? If so, does the structure have any vulnerable design features?	<u>/es_</u>
<ul> <li>a. Piers, abutments with <u>spread footings</u> or short pile foundations.</li> <li>b. Superstructures with <u>simple spans</u> or non-redundant support systems.</li> <li>c. Inadequate waterway opening.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	
3. Are any characteristics of an aggressive stream or waterway present?  **Active degradation or aggredation of streambed.** b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other.	- Yes
4. Is bridge located on stream reach with any adverse flow characteristics?	
<ul> <li>a. Crossing near stream confluence.</li> <li>b. Crossing of tributary stream near confluence with larger streams.</li> <li>c. Crossing on sharp bend in stream.</li> <li>d. Location on alluvial fan.</li> <li>e. Other.</li> </ul>	
E Other comments or observations	_

# NORTHFIELD BROOK LAKE BRIDGE ON OLD ROUTE 254 (UPPER), THOMASTON, CT FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 24 August 1993

DATE OF PREVIOUS INSPECTIONS: In-depth, Dec 84

Routine, Aug 87 Routine, Aug 89 Routine, June 91

#### RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	19T	28T	
3	34T	52 <b>T</b>	
3S2	49T	52T	

#### **EVALUATION** (See attached "Structures Inspection Field Report")

A. Roadway & Railings

The overall condition of the deck and railings is good (condition 7). The upper cable of the north east approach is loose and tangled. The remaining cable guardrails along the roadway are in very good condition. Both bridge railings are in good condition. Most concrete spalls have been patched. One repair in the south end of the east rail has spalled due to wood forming remaining in the patch. The deck and approaches are in good condition. Various areas of the deck appear to have been filled with bituminous patching.

B. Fascias and Curbs

The condition of the fascias and curbs is good. The concrete shows evidence of abrasion typical of aged concrete. Of minor concern is the lack of joint filler between curb monoliths. There is a buildup of debris in some of these joints.

C. Underside of Deck

The underside of the deck is in very good condition (condition 8) and appears to have been recently painted. The bearings and underside of the concrete deck are

in good condition with no signs of distress or deterioration.

D. Wingwalls and Abutments

The wingwalls and abutments are in good condition (condition 7).

Moderate abrasion is typical of all walls. One minor vertical crack was noted in the south east wingwall and minor efflorescence noted on the west walls. The weep holes in the south abutment are clear and appear to be functioning properly. The weep holes in the north abutment were buried under sand deposited against the wall.

E. Channel

The channel is undergoing various amounts of erosion. Although no scour below the bridge footings was noted, moderate aggredation was present along the north abutment. Both upstream and downstream of the bridge, dense vegetation was encroaching upon the channel.

CONDITION RATING

In-depth 7
Interim 1987 7
Interim 1989 7
Routine 1991 8
Routine 1993 7

#### RECOMMENDATIONS

#### Status of Previous Recommendations

No Previous recommendations

#### Revised Recommendations

The deficiencies noted are not of much concern at this time. They may be combined with repairs to other local bridges in the future.

IA=INACCESSIBLE

## STRUCTURES INSPECTION FIELD REPORT

### **ROUTINE INSPECTION**

	A = \	_		bridge dep	t. no.	8-structu	re no. EDCT 091 0004	90-date inspected 8/23/93
2-dist.	ASTON C 104-highway sys		22-owner	, 000.	27-year b	ouilt	106-year rebuilt	11-milepoint
43-structure					quality control engineer			
STEE		- SPANJ WI	DE FLANK	OF BEAM				
07-facility ca					team lea			
	D RT 25.	4				JOE COLUCCI		
06-features					M DESCHELES, M WALSH, M TORID			
NOR	THEIELD .	E2004			IN DE	3CHUA!	5, M WALSHI	M IORIO
item 58 DECK		7	item 59 SUPERST	RUCTURE		B	item 60_ SUBSTRUCTURE	7
<ol> <li>We</li> <li>Dec</li> <li>Sta</li> <li>Cui</li> <li>Me</li> <li>Sid</li> <li>Pai</li> <li>Ra</li> <li>Ani</li> <li>Dra</li> <li>Lig</li> <li>Uti</li> <li>De</li> </ol>	edian dewalks rapet illing ti Missile Fence ains ghting Standards	7 7 7 7 7 7 7 7 7 7 7 7 7 8	2. Stringe 3. Diaphr 4. Girder 5. Floor E 6. Trusse 7. Rivets 8. Welds 9. Collisi 10. Load I 11. Memb 12. Load V 13. Paint- 14. Year F	ragms s or Beams Beams or Bolts on Damage Deflection er Alignmen Vibration Epoxy Painted Clearance	ft -		1. Abutments a-Wings b-Backw. c-Bridge d-Breast e-Footing f-Piles g-Erosio h-Settler 2. Piers or Bents a-Caps b-Colum c-Web d-Footin e-Piles f-Scour g-Settler 3. Collision Dama 4. Hydraulic-Aded	Seats wall gs 7 n ment g g ment g ment g ment g ment g ment
Actual Po	esting	H 3 3S2	Single		Overhe	ad Signs yes	(attached to bridge)	
Recomme From Rat	ended Posting ting Book				1. Weld		NA	
SIGNS II Y or N	N PLACE	at bridge	advar N		3. Con		MA	
LEGIBIL	ITY	N	N		Item93	b U/W I	nspection Date: No	NE
ITEM 61-channel and channel protection 7				raffic Sa	fety features	condition		
hannel scour 2. embankment erosion 3. fender system 4. spur dikes & jetties   5. rip rap or slope paving 6. effectiveness 7 2. transitions 7 3. approach guardrail 7 4. guardrail terminal 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7								

X=UNKNOWN NA=NOT APPLICABLE

PROJECT: NORTH FIELD BROOK LAKE
NAME: LIPPER RT 254 BRIOLE
LOCATION: OLD RT 254 (UPPER)

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	YES			
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES			
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructure with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway openings.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	YES YES NO NO YES			
3. Are any characteristics of an aggressive stream or waterway present?	YES			
<ul> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	VES NO NO			
4. Is the bridge located on a stream reach with any adverse flow characteristics?				
<ul> <li>a. Crossing near stream confluence.</li> <li>b. Crossing of tributary stream near confluence with larger streams.</li> <li>c. Crossing on sharp bend in stream.</li> <li>d. Location on alluvial fan.</li> <li>e. Other.</li> </ul>	NO NO NES			
5. Other comments or observations.				

# NORTHFIELD BROOK LAKE BRIDGE ON OLD ROUTE 254, (LOWER) THOMASTON, CT FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 24 August 1993

<u>DATE OF PREVIOUS INSPECTIONS</u>: In-depth, Dec 84

Routine, Aug 87 Routine, Aug 89 Routine, Sept 91

#### RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	16T 39T	23T 55T	
3S2	62T	86T	

#### EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway, & Railings

The overall condition of the roadway, railings and approaches is good. The wearing surface of the deck has been recently replaced. Cracks were noted across the deck at approximately 8 to 10 foot intervals. The cracking appears to be the result of improper curing. The cable roadway guardrails are in good condition. One cable along the south east approach is loose. The bituminous approaches have been repaired recently. The new approaches are slightly higher than the deck causing a slight impact when entering and exiting the bridge. The railings at each approach are in good condition. The west guardrail shows some abrasion of the concrete, typical of its age.

- B. Curbs and Fascias
- The curbs and fascias along both sides of the deck are in good condition with no apparent signs of distress or deterioration.
- C. Underside of Deck

The overall condition of the superstructure below the deck is good. Three of the T-beams on the

east side of the bridge have minor spalls and minor to moderate efflorescence. The two inner T-beams are in very good condition. The two west steel beams are in good condition. There is minor rusting apparent on the underside of the deck from the reinforcement chairs. The bearings for both the steel and concrete beams are in good condition.

D. Wingwalls and Abutments

The wingwalls and abutments are in good condition. Only minor cracking and efflorescence was noted along the wingwalls.

E. Channel

The bridge is located at the end of a bend in the channel. There is some aggredation of the channel along the north abutment. The downstream side of the channel is clear.

CONDITION RATING

In-depth 7
Routine 1987 7
Routine 1989 6
Routine 1991 8
Routine 1993 7

#### RECOMMENDATIONS

#### Status of Previous Recommendations

No previous recommendations.

#### Revised Recommendations

No new recommendations at this time.

## ROUTINE INSPECTION

	bridge dep			90-date inspected
2-dist. 104-highway system	22-owner	27-year built	106-year rebuilt	24 AVSJ 93 11-milepoint
Non- FEDERAL.	COT.	1936		•
43-structure type		quality control eng		
SIMPLE SPAN CONCRETE T-BEAMS,	/ STEEL STRINGER	NICK	FOREES	
07-facility carried		team leader		
OLD ROUTE 254 (Lou O6-features intersected	NER)	team members		
NORTHFIELD BROK.		M. DESCHE	F. M. WALSH.	MJORIO
NORTH HTTEOD ONDE	T .	101111111111111111111111111111111111111		
item 58	item 59	7	item 60	7
DECK	SUPERSTRUCTURE		SUBSTRUCTURE  1. Abutments	
1. Wearing Surface	1. Bearing Devices		a-Wings	一
2. Deck-Condition	2. Stringers		b-Backw	
3. Stay in Place Forms	3. Diaphragms		c-Bridge	
	4. Girders or Beams	<u> </u>	d-Breast	wall 🗍
00.00	5. Floor Beams		e-Footing	gs 7
5. Median	6. Trusses		f-Piles	
6. Sidewalks 7. Parapet	7. Rivets or Bolts		g-Erosio h-Settler	
7	8. Welds		2. Piers or Bents	
8. Railing	9. Collision Damage		a-Caps	=
9. Anti Missile Fence	10. Load Deflection		b-Colum	n 🖃
10. Drains	11. Member Alignmer	nt 🖴	c-Web	
11. Lighting Standards	12. Load Vibration		d-Footin e-Piles	g _
12. Utilities	13. Paint-Epoxy	- <u>- 2</u>	e-Piles f-Scour	
13. Deck Joints	14. Year Painted		g-Settle	
14. Approach Settlement	15. Under Clearance		3. Collision Dama	age <u>-</u>
	Clearance Signs	yes HA no	4. Hydraulic-Ade	quacy
	Cila	Overhead Signs	(attached to bridge)	
Actual Posting H 3 3S2	Single	yes	no	
Recommended Posting From Rating Book	-	1. Welds		
		2. Bolts	-	
SIGNS IN PLACE at bridge	advance		-	
Y or N	N	3. Condition		
LEGIBILITY	-			
		Item93b U/W I	Inspection Date:	
ITEM 61-channel and channel protection 7				
nannel scour	1. bridge railing			
2. embankment erosion $\mathcal{S}$ 6. eff	2. transitions		<u> </u>	
3. fender system 7. de	bris 7	3. approach		
4. spur dikes & jetties 8. ve	getation 7	4. guardrail t	erminal L	
X-UNKNOWN	NA=NOT APPLIC	CABLE	JA=INAC	CESSIBLE

PROJECT: NORTH FIELD BROOK NAME: OLD RT 254 (LOWER) LOCATION: OLD RT 254

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	YES
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructure with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway openings.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	Yes Ves Vo No Yes
3. Are any characteristics of an aggressive stream or waterway present?	Yes
<ul> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	765 20 20 20
4. Is the bridge located on a stream reach with any adverse flow characteristics?	YES
a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other.	NO NO
5. Other comments or observations.	غائم و له

# BLACK ROCK LAKE OLD NORTHFIELD ROAD BRIDGE, THOMASTON, CT FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 24 August 1993

<u>DATE OF PREVIOUS INSPECTIONS</u>: In-depth, Dec 84

Routine, Aug 87 Routine, Aug 89 Routine, June 91

#### RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	11T	16T	Ratings similar to
3	25T	4 OT	those determined in
<b>3</b> S2	39 <b>T</b>	63T	the 1984 in-depth
3-3	49T	78T	report.

#### EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway, Railings, and Deck.

The general condition is good (condition 8). The bituminous wearing surface on the north approach and south approach is in good condition. The transitions to the deck from the north and south approaches are not smooth. The expansion joint is sealed and in adequately good condition. The concrete bridge deck is in good condition. The scuppers are clear. The rails on the bridge deck are in good condition. There is some minor vegetation growth at the curbs on the bridge deck. The approach guardrails are in good condition.

B. Fascia and Curbs

The general condition is good (condition 8). The overall condition of the curbs is good; they have recently been painted. There is a minor crack at the northeast corner of the curb and some minor honeycombing.

C. Underside of Deck and Bearings. The overall condition is good (condition 8). The underside of the deck is in good condition. The girders are in good condition with no signs of rust. The bearings appear to be well seated and in good condition.

D. Wingwalls and Abutments

The overall condition is good (condition 7). The granite block wingwall on the southwest side has some cracked mortar with vegetation growth in the cracks.

The other wingwalls are in good

condition. The abutments are in good

condition.

E. Channel

The channel is in good condition (condition 8). There is heavy vegetation upstream and downstream.

CONDITION RATING

Previous in-depth: 7 Interim 1987: 7 Interim 1989: 7 Routine 1991: 8 Routine 1993: 8

### RECOMMENDATIONS

### Status of Previous Recommendations

There were no previous recommendations.

### Revised Recommendations

Repair cracked mortar on southeast wingwall.

Estimated Cost \$1,000

## STRUCTURES INSPECTION FIELD REPORT

### **ROUTINE INSPECTION**

THOMASTON, G			bridge dep	t. no.	8-structu	re no. 2500, TOF 19006	90-date inspect	ed
-dist. 104-highway system		22-owner CENE	D	27-year	built رايدن	106-year rebuilt	11-milepoint	
3-structure type	IDS FLA	NG BA	m	quality control engineer  NICK FORBES				
7-facility carried  OUD NORTH	FIELD	ROAD		team le	ader <u></u>	SEPH COLU		
6-features intersected NOETH FIELD	BROOK				nembers	M. DESC. HENES	M. WALSH	
item 58	8	item 59	o		3	item 60_ SUBSTRUCTURE	8	
DECK			RUCTURE		<b>P</b>	1. Abutments	-	
1. Wearing Surface	8		g Devices		8	a-Wings	7	
2. Deck-Condition	3	2. Stringe			8	b-Backwa		
3. Stay in Place Forms	NA	3. Diaphi			8	c-Bridge		
4. Curbs	8		s or Beams		8	d-Breast		
5. Median	NA	5. Floor I			WA	e-Footing f-Piles	gs WA	
6. Sidewalks	NA	6. Trusse			80	g-Erosio		
7. Parapet	NA		or Bolts		NA	h-Settlen	nent 8	
•	8	8. Welds			NM	2. Piers or Bents		
8. Railing	NA		ion Damage Deflection		8	a-Caps	n Na	
9. Anti Missile Fence	8		per Alignmer	.+	8	b-Colum c-Web	n NA	
10. Drains		12. Load	_	ıı		d-Footing	<b>F</b>	
11. Lighting Standards	PA	12. Load			8	e-Piles	<u> </u>	
12. Utilities	NK	14. Year			91	f-Scour	NA	
13. Deck Joints	7		r Clearance	ft	in	g-Settler	c. for	
14. Approach Settlement	7		ce Signs		es 💢 no	3. Collision Dama		
				Overh	ead Signs	(attached to bridge)		
Actual Posting H	3 352 NA NA	Single NA			yes	X no		
Recommended Posting From Rating Book	NA NA	HI		1. We	lds			
SIGNS IN PLACE at brid	ge	adva	_ 1	<ol> <li>Bol</li> <li>Cor</li> </ol>	ts ndition			
LEGIBILITY -		_	-	Item9	3b U/W	Inspection Date:		
ITEM 61-channel and chann		on 7 ap or slope p	paving 114	,	Traffic Sa	ifety features	condition	
<ol> <li>embankment erosion</li> <li>fender system</li> </ol>	8 6. effe ₩ 7. deb	ctiveness	Baving B	2. t	ransitions approach guardrail t	guardrail 🔐	8 8	

JA-INACCESSIBLE

X-IINKNOWN

PROJECT: BLACK LOCK LAKE
NAME: OLD NORTHNEY LD BRIDGE
LOCATION: THOMASTON, CT

# BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	No
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructure with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway openings.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	No YES No No YES No
3. Are any characteristics of an aggressive stream or waterway present?	YES
<ul> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	YES No No No No No
4. Is the bridge located on a stream reach with any adverse flow characteristics?	No
<ul> <li>a. Crossing near stream confluence.</li> <li>b. Crossing of tributary stream near confluence with larger streams.</li> <li>c. Crossing on sharp bend in stream.</li> <li>d. Location on alluvial fan.</li> <li>e. Other.</li> </ul>	No No No No
5. Other comments or observations.	No

# HOP BROOK LAKE BRIDGE ON OLD ROUTE 63, MIDDLEBURY, CT FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 24 August 1993

DATE OF PREVIOUS INSPECTIONS: In-depth, Dec 84

Routine, Sept 87
Routine, Aug 89
Routine, Sept 91

### RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	23T	32T	The 8 ton rating suggested in the 1984 in-depth inspection can be increased to the full inventory capacity since the deteriorated concrete of the arched section has been satisfactorally repaired.
3	38T	54T	
3S2	55T	77T	
3	61T	86T	

### EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway, Railings, and Deck.

The general condition is good (condition 7). The bituminous wearing surface on the north approach and south approach has some minor rutting. There are slight depressions at the transitions to the deck from the north and south approaches. The wearing surfaces on the north and south approaches have some minor rutting. Small stones from a chip seal have been left along the curb. The rails on the bridge deck have been recently patched are in good condition. The approach quardrails are in good condition.

B. Fascia and Curbs

The general condition is good (condition 8). The overall condition of the curbing is good. The curbs have recently been patched with concrete; however, the concrete has some minor surface deterioration. The fascias are in very good condition.

C. Underside of Deck and Bearings.

The overall condition is good (condition 7). The arched section has been recently repaired and has a new coating of "shot-crete". The coloring of the "shot-crete" is inconsistent and varies from very light gray to dark gray. The tee beams on the west side are in good condition with some minor honeycombing. The tee beams on the east side have a few spalls and minor honeycombing.

D. Wingwalls and Abutments

The overall condition is good (condition 7). The north and south abutments are in good condition. The weepholes on the south abutment are clear. The weepholes on the north abutment are buried by aggredation. The wingwalls are in good condition; however, there is miscellaneous vegetation growing in front on them.

E. Channel

The channel is in good condition (condition 7). The bridge is located on a bend in the river. This is causing aggredation along the northern abutment and creates the possibility of scour along the southern abutment. There is a confluence just west of the northern abutment.

### CONDITION RATING

Previous in-depth: 7
Interim 1987: 7
Interim 1989: 5
Routine 1991: 5
Routine 1993: 7

### RECOMMENDATIONS

### Status of Previous Recommendations

Remove trees and vegetation in front of wingwalls.

Not done

### Revised Recommendations

Implement the previous recommendation.

Total Estimated Cost

\$5000

### STRUCTURES INSPECTION FIELD REPORT

### **ROUTINE INSPECTION**

MIDDLEBURY, CT	bridge dep		no. SEDCTOS 10007	90-date inspected 8 24 93		
2-dist. 104-highway system	22-owner CENED	27-year built 10	06-year rebuilt 1944	11-milepoint		
43-structure type	dole T-	quality control engine	er JICK FORBES			
07-facility carried	onc. TEE	team leader				
OLD ROUTE 62		team leader JOSEPH COUKL				
06-features intersected	ACCESS	team members	1. Describes a	۸ معدد ۱۸		
Hop Brook		P. Dele P	DESCRIBES I			
item 58  DECK  1. Wearing Surface  2. Deck-Condition	SUPERSTRUCTURE  1. Bearing Devices  2. Stringers	ya ya	item 60_ SUBSTRUCTURE  1. Abutments a-Wings b-Backwa	8		
2. Deck-Condition 3. Stay in Place Forms 4. Curbs 7	Diaphragms     Girders or Beams	7	c-Bridge S d-Breastv	Seats B		
5. Median  6. Sidewalks	5. Floor Beams 6. Trusses 7. Rivets or Bolts	MA NA NA	e-Footing f-Piles g-Erosion	[A]		
7. Parapet 8. Railing	8. Welds 9. Collision Damage	NA NA	h-Settlem 2. Piers or Bents a-Caps	ent &		
9. Anti Missile Fence  10. Drains  11. Lighting Standards	<ul><li>10. Load Deflection</li><li>11. Member Alignmer</li><li>12. Load Vibration</li></ul>	nt &	b-Column c-Web d-Footing	NA		
12. Utilities	13. Paint-Epoxy 14. Year Painted	NA AU	e-Piles f-Scour			
13. Deck Joints  14. Approach Settlement	15. Under Clearance Clearance Signs	tt in yesno	g-Settlen 3. Collision Dama 4. Hydraulic-Adeq	ge <b>NA</b>		
	17 April	8				
Actual Posting H 3 3S2	Single	Overhead Signs (at	tached to bridge)  no			
Recommended Posting From Rating Book	9	Welds    Bolts				
SIGNS IN PLACE at bridge Y or N	advance	3. Condition	_			
LEGIBILITY		Item93b U/W Insp	pection Date:			
ITEM 61-channel and channel protect	ion 7	36-Traffic Safet	•	condition		
2. embankment erosion 7 6. eff. 3. fender system 7 7. de	rap or slope paving ectiveness 7 bris 7 getation	1. bridge railing 2. transitions 3. approach guardrail terr	ardrail			
X=UNKNOWN	NA=NOT APPLIC	ABLE	JA=INACC	ESSIBLE		

PROJECT: Hoy Brook LAKE
NAME: OLD RT 63 BRIDGE
LOCATION: MIDDLEBURY CT

# BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	No
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	Yes
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructure with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway openings.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	No Yes No No No
3. Are any characteristics of an aggressive stream or waterway present?	YES
<ul> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	YES NO NO NO
4. Is the bridge located on a stream reach with any adverse flow characteristics?	YES
<ul> <li>a. Crossing near stream confluence.</li> <li>b. Crossing of tributary stream near confluence with larger streams.</li> <li>c. Crossing on sharp bend in stream.</li> <li>d. Location on alluvial fan.</li> <li>e. Other.</li> </ul>	У <u>е</u> ѕ
5. Other comments or observations.	No

### TULLY LAKE DOANE HILL ROAD BRIDGE, ROYALSTON, MA FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 24 June 93

<u>DATE OF PREVIOUS INSPECTIONS</u>: Inventory, 24 September 84 Routine, 15 September 87 Routine, 7 September 89 Routine, 11 July 91

#### RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	13.5T	25.4T	No change in ratings
3	16.0T	30.1T	
3S2	24.7T	46.7T	
3-3	31.0T	57.6T	

### EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway and Railings: Overall condition 6. A New tar and gravel surface coat has recently been applied to the road and the bridge. When placing this coat, however, several of the vertical deck drains were covered and are now blocked. The new surface coating also continued across the joints in the deck. The new surfacing was not compacted well as it approached the openings in the curbings and therefore makes these openings ineffective for drainage. The extensive vegetation growth in the openings also creates an obstruction to the proper drainage of the deck. Weight limit signs were not present. The 3"x8" timber rails which are dried out and brittle are loose and inadequate. and are loose to the touch. The cable quard rails at the approaches to the deck are in good condition, however, they are very loose and need to be tightened and repaired.

#### B. Curbs & Fascias:

Overall condition 6. There is extensive spalling and wear on both curbs. drainage openings, as previously mentioned, are mostly filled with

vegetation and debris. There is extensive spalling and efflorescence along the exterior fascias of the bridge.

C. Underside of Deck:

Overall condition 7. Minor spalling around deck drains was noted. Most of the structural steel exhibited moderate rusting. The exterior beams show the greatest amount of rust. The beam on the interior of the north face of the bridge which has been noted as not having enough clearance, has not yet been cut. It is recommended that this beam be cut in order to allow 2" to 2 1/2" of clearance from the face of the abutment. The bearings are in good condition with minor rust and debris buildup.

D. Wingwalls/Abutments:

Overall condition 8. The wingwalls and abutments are in good condition. Bonding and alignment are good. The walls show no signs of distress.

E. Channel:

The overall condition is 8. The water flows smoothly through the channel with little or no debris buildup. Some minor abrasion was evident at the base of the abutments below the flow line.

#### CONDITION RATING

Inventory, 1984 7
Routine, 1987 7
Routine, 1989 7
Routine, 1991 7
Routine, 1993 7

### **RECOMMENDATIONS:**

### Status of Previous Recommendations

 Repair loose guard rail cables on northeast approach; repair detached upper guardrail cable on southwest approach; replace timber bridge rail with steel tubular section. Estimated cost \$7000.

Not done

2. Clear debris from fascia openings and patch spalled areas with polymer modified repair mortar. Estimated cost \$3000.

Not done

3. Clean all debris and vegetation from gutters. Repair pavement on approaches and deck by cold

Not done

planing 1" from existing and repaving; clean deterioration from around drains; compact new material around drains prior to repaving. Estimated cost \$5000.

4. Clean and paint all structural steel and bearings. Cut or burn web and bottom flange of first interior beam (North side, east abutment) as required to re-establish a minimum clearance of two inches. Estimated cost \$15000. Not done

### Revised Recommendations

1. Repair loose guardrail cables on northeast approach; repair detached upper guardrail cable on southwest approach; replace timber bridge railing with new railing.

Estimated cost \$7000.

2. Clear debris from fascia openings and patch spalled areas with polymer modified repair mortar.

Estimated cost \$3000.

- 3. Clean all debris and vegetation from gutters. Can be done by project personnel.
- 4. Clean and paint all structural steel and bearings. Cut or burn web and bottom flange of first interior beam (North side, east abutment) as required to re-establish a minimum clearance of two inches.

Estimated cost \$15000.

Total Estimated Cost \$25000.

### STRUCTURES INSPECTION FIELD REPORT

### **ROUTINE INSPECTION**

c.				bridge dep	t. no.	8-structu	re no.	90-d	ate inspected
ROYAL	STON LINE	-TULLY	LAKE	COE			EDMA 2510010		24/93
2-dist.	104-highway s		22-owner		27-year		106-year rebuilt		ilepoint
	NOW. FE	DERAL	COE		19			$\alpha$	01800
43-structure			01 6 3	5000	quality control engineer  NICK FORSES				
07-facility ca		ANGE BEAM	will coll	DECK	team le		FOLDES		
Do An		POND				SEPH	Comiller		
06-features					1	nembers			
TUL	y PIVE	2			H.	Desci	HENES F. FUNC	<u></u>	
H FO			item 59	, , , , , , , , , , , , , , , , , , , ,		7	item 60		8
item 58	_	6		RUCTURE			SUBSTRUCTURE	Ξ	$\mathcal{C}_{\mathcal{C}}$
DECK				g Devices		7	1. Abutments		_
1. We	aring Surface			•		44	a-Wings		E
2. Dec	k-Condition	7	2. String			7	b-Backw		8
3. Sta	y in Place Form	s A	3. Diaph	_		7	c-Bridge		B B
4. Cur	bs	6		s or Beams		127	d-Breast		P.K.
5. Mei	dian	NA	5. Floor			N *	e-Footing	ys	PA
	ewalks	NA	6. Truss			7	g-Erosio	n	3
		6	7	or Bolts			h-Settler		8
7. Par			8. Welds			NA	2. Piers or Bents	3	
8. Rai	ling			ion Damage			a-Caps		14
9. Ant	i Missile Fence	12	,	Deflection			b-Colum	าท	AC
10. Dra	ains	عا	11. Memb	er Alignmer	it	8	c-Web		NV
11. Lig	hting Standards	HL	12. Load	Vibration		×	d-Footin	ng	ag ag
12. Util	iities	Ais	13. Paint-	-Ероху		ط	e-Piles f-Scour		ria Pia
13 De	ck Joints	7	14. Year	Painted			0 - 111 -		NA
	proach Settleme	ent 8	15. Unde	r Clearance		NA in	3. Collision Dama		$\boxtimes$
14. Ap	proach Settleme		Clearan	ce Signs	yes NA no 4. Hydraulic-Adequacy			8	
			O: - 1		Overh	ead Signs	(attached to bridge)		
Actual Po	sting	H 3 3S2	Single	9		yes	no		
	ended Posting	:3 lb 25			1. Wel	lds	[nv]		
From Rat	ing book	لتتا لين يي		,	2. Bolt	te	NA		
SIGNS	N PLACE	at bridge	adva	nce	Z. DOII	15	134		
Y or N	11 L/10L	N	Nos	_	3. Cor	ndition	NN		
			12/						
LEGIBIL	ITY	NA .	101	<u> </u>	Item9	3b U/W	Inspection Date:	NE-	
ITEM	61-channel an	d channel protect	ion 🗀		36-	Traffic Sa	ifety features		
TI EIVI	or originate all				,		36		condition
	nnel scour		rap or slope	oaving No	1. b	ridge rail	ing <u>c</u>		6
1	oankment eros		ectiveness	NA	1	ransitions		╡	7
	der system r dikes & jettie		oris getation	3		ipproacn Juardrail t	gaararan		1
7. Spu	. Since a jettle	U [[2/-] 0. VO	901411011		7.9	Juan di dil 1			

NA=NOT APPLICABLE

X=UNKNOWN

IA=INACCESSIBLE

PROJECT: TULL! LAKE

NAME: DOPAE HILL ROAD

LOCATION: ROYALSTON MA.

# BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	No
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	yes
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructure with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway openings.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	yes yes no no
3. Are any characteristics of an aggressive stream or waterway present?	<u> </u>
<ul> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	20 20 20 20 20
4. Is the bridge located on a stream reach with any adverse flow characteristics?	No
<ul> <li>a. Crossing near stream confluence.</li> <li>b. Crossing of tributary stream near confluence with larger streams.</li> <li>c. Crossing on sharp bend in stream.</li> <li>d. Location on alluvial fan.</li> <li>e. Other.</li> </ul>	No   No   No   No
5. Other comments or observations.	None

# EVERETT LAKE CHOATE BROOK FY93 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION: 9 Sept 93

DATE OF PREVIOUS INSPECTIONS: 31 July 91 Routine

11 Sep 89 Routine 17 Sep 87 Routine 25 Mar 85 In-depth

RATING (T = TONS)

Type	Inventory	Operating	Comments
H	2.0 T	4.4 T	Ratings from 1985
3 3S2	3.6 T 5.7 T	7.9 T 12.5 T	in-depth.
304	5./ 1	12.5 1	

Note: Ratings are estimated for H-20 loading for the new concrete deck for 1993 and final calculation will be performed within FY94.

### EVALUATION (see attached field report)

A.	Approaches	Overall rating is 6. Guard rails are new but only 25' long on east side and no erosion control on both sides.
В.	Bridge Deck	Overall rating is 7. New bridge deck with guard rails on both sides. Missing bolts were located on the middle of the south guard rails. Most of the I-beams posts do not line up their centerline axis.

### C. Substructure

Overall rating is 7. At the northeast abutment corner, a one and half foot deep scour is located. There are honey comb and hairline cracks at the southeast bridge abutment. Tree branches and debris are built up on the southside of the bridge deck.

CONDITION RATING:	Previous in-depth:	6
	Routine 1987:	6
	Routine 1989:	5
	Routine 1991:	4
	Routine 1993:	7

### **RECOMMENDATIONS:**

### Recommendations

1. The length of the guardrail for the eastside approach should be increased another 25 feet due to the sharp curve and deep drop at the edge.

Estimated cost: \$ 1500.00

2. There should be some erosion control on the embankments along both side approaches.

Estimated cost: \$ 2000.00

3. The project personnel should remove the tree branches and debris under or near the bridge deck.

Total estimated cost: \$ 3500.00

### STRUCTURES INSPECTION FIELD REPORT

### **ROUTINE INSPECTION**

Cny	WEARE	N.H .		bridge dep	t. no.	8-structu	re no.	90	o-date inspected
2-dist.	104-highway s	ystem	22-owner Corps 9	ENGR		20	106-year rebuilt 1993	11	-milepoint
43-structure	type Concret	E SLAB S	INGLE SI	OAN	quality c	ontrol eng			
07-facility ca	arried RECK	EATION AR	EA ALLES	is Rd	team le	JOE	COLUCCI		
06-features		HOATE &	BRODK			nembers	ENES /F. F	un 4	
item 58	- A straight region	7	item 59			7	item 60_ SUBSTRUCTU	IDE	7
DECK				RUCTURE		7	1. Abutments	INE	
1. Wea	aring Surface	8		g Devices		4/0	a-Win	gs .	7
2. Dec	k-Condition	8	2. Stringe 3. Diaphr			NA	b-Bac		7 8
3. Stay	y in Place Form			s or Beams		NA	1	ge Seat astwall	s 8 7
4. Cur	bs	8	5. Floor f			2	e-Foo		7
5. Med	dian	NA	6. Trusse	-		NA	f-Piles	•	20/1
6. Side	ewalks	MA	7. Rivets	or Bolts		7	g-Ero		6
7. Par	apet	M	8. Welds			NA	h-Set	tiement	PA
8. Rail	ling	7	9. Collisi	on Damage		NA	a-Car		NA
9. Ant	i Missile Fence	WA	10. Load I	Deflection		MA	b-Col		1/1
10. Dra	ins	ME	ì	er Alignmen	it	8	c-Wel	b ·	1/4
11. Ligi	hting Standards	116	12. Load	Vibration		NA	d-Foo	_	<u> </u>
12. Util	ities	NA	13. Paint-			<u>NE</u>	e-Pile f-Sco	_	MA
13. Dec	ck Joints	NA	14. Year I		NA	<u>M</u>		tlement	MA
14. Apr	oroach Settleme	ent NA		Clearance -	ft ft	in	3. Collision Da		NA
			Clearand	ce Signs	ye	es no	4. Hydraulic-A	dequacy	/ VF
Actual Pos	sting	H 3 3S2	Single		Overhe	ead Signs yes	(attached to bridge)		
			N/A						
Recomme From Rati	ended Posting ng Book		20		1. Wel		NA		
SIGNS IN Y or N	N PLACE	at bridge	advar N	3	2. Bolt 3. Con		NA		
LEGIBILI	TY	NA	NA		Item93	Bb U/W I	nspection Date:	NON	E
ITEM 6	31-channel and	d channel protecti	ion 6		36-7	raffic Sa	fety features	00	
2. emb 3. fend	nnel scour eankment eros ler system dikes & jetties	ion 6. effe VA 7. del	rap or slope p ectiveness bris getation	aving 6 6 7	2. tr	ridge raili ansitions pproach ( uardrail to	ng guardrail erminal	36 <b>ク</b> シ ル	condition  7  MA  MA

NA-NOT APPLICABLE

X-UNKNOWN

JA-INACCESSIBLE

PROJECT: EVERETT LAKE

NAME: CHOATE BROOK

LOCATION: WEAKE, N.H.

# BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	YES_
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructure with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway openings.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	YES YES NO YES NO
3. Are any characteristics of an aggressive stream or waterway present?	YES
<ul> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	NO NO YES YES UNKNOWN
4. Is the bridge located on a stream reach with any adverse flow characteristics?	
<ul> <li>a. Crossing near stream confluence.</li> <li>b. Crossing of tributary stream near confluence with larger streams.</li> <li>c. Crossing on sharp bend in stream.</li> <li>d. Location on alluvial fan.</li> <li>e. Other.</li> </ul>	No No No No
5. Other comments or observations.	NONE

# OTTER BROOK LAKE EXIT BRIDGE, KEENE, N.H. FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 18 August 1993

DATE OF PREVIOUS INSPECTIONS: In-depth, \*.

Routine, Sept 87 Routine, Sept 89

Routine, 22 August 1991

### RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	18.0T	32.6T	Load capacities
3	22.1T	39.9T	recalculated for
3S2	34.4T	62.1T	prestressed beams

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway, Railings, and Deck.

The overall condition is good (condition 7). The bituminous wearing surface onthe deck is in good condition. The south approach surface is in good condition with a minor crack at the transition to the bridge deck. The north approach has some minor rutting along wheel lines and a crack at the transition to the bridge deck. The terminal unit of the guardrail in the northwest corner is damaged. The design of the existing terminal unit in this location is poor and should be extended around the corner and buried. The southeast top railing is loose.

B. Fascia and Curbs

The overall condition is good (condition 7). Both the curbs and fascias have hairline cracks approximately every two feet. There is also some spalling at the caps covering the transrverse posttensioned reinforcing. Minor debris and vegetation is collecting along the curbing.

C. Underside of Deck and Bearings.

The overall condition is good (condition 7). The underside of the deck is in good condition. There is some minor leakage of water from the deck onto the south abutment. No problems were noted with the bearings.

with the bearings.

D.	Wingwalls	The	overall	condition	is	fair	(condition
	and Abutments	6).	The nort	th abutment	- is	s in a	poor

condition. The northeast footing has a spall measuring two foot by two foot by six inches. There is also evidence of scour and erosion along the northeast

wingwall.

E. Channel The overall rating is 5. The water is deepest along the abutments. The north

east abutment is scoured and deteriorated as noted on previous reports. The channel contains many rocks and has the potential to collect

debris.

CONDITION RATING Interim 1987: 7

Interim 1989: 6 Routine 1991: 6 Routine 1993: 6

### RECOMMENDATIONS

### Status of Previous Recommendations

Cost Est Status

1.	Repair erosion and deteriorated	\$20,000	Not Done
	concrete at the base of the abutments.		

2. Provide stone apron at abutment \$15,000 Not Done as scour remedial action.

3. Remove vegetation from wingwalls \$500 Not Done and curbs.

Total \$35,500

### Revised Recommendations

Implement above recommendations.

Extend and bury northeast \$1500 guardrail terminal unit.

Total Updated Estimated Cost \$37,000

### STRUCTURES INSPECTION FIELD REPORT

### **ROUTINE INSPECTION**

FEBNE N.H		bri	dge dep		ure no. PNEDNH 3310009	90-date inspected
2-dist. 104-highway syste	m	22-owner CoE		27-year built 19 6 7	106-year rebuilt	11-milepoint
43-structure type  PRESTRESSED CAN  07-facility carried  REC AREA EX	CRETE BI	4046 BEA	ıms	quality control en	<del>-</del>	
06-features intersected				team members		e Describes
item 58 DECK  1. Wearing Surface 2. Deck-Condition 3. Stay in Place Forms 4. Curbs 5. Median 6. Sidewalks 7. Parapet 8. Railing 9. Anti Missile Fence 10. Drains 11. Lighting Standards 12. Utilities 13. Deck Joints 14. Approach Settlement	7 7 2 2 2 2 7 2 2 2 7 7 7	item 59 SUPERSTRUC  1. Bearing De 2. Stringers 3. Diaphragm 4. Girders or 5. Floor Bear 6. Trusses 7. Rivets or E 8. Welds 9. Collision D 10. Load Defle 11. Member A 12. Load Vibra 13. Paint-Epox 14. Year Paint 15. Under Cle Clearance Si	evices  Beams  Bolts  amage ection lignment ation  ky ed arance _	S Na Na	<ol><li>Collision Dama</li></ol>	Seats  Seats  Vall  Seats  A  A  A  A  A  A  A  A  A  A  A  A  A
Recommended Posting From Rating Book	3 3S2	Single  NA  advance		1. Welds 2. Bolts 3. Condition	(attached to bridge)  NA  Inspection Date:	<b>ル</b> モ
'TEM 61-channel and charannel scour 2. embankment erosion 3. fender system 4. spur dikes & jetties	<b>5</b> 5. rip r	ap or slope pavin ctiveness ris	7777	36-Traffic Sa  1. bridge raili 2. transitions 3. approach ( 4. guardrail to	fety features  ng  Jak guardrail	condition 7 7 7 7 7

NA-NOT APPLICABLE

IA-INACCESSIBLE

PROJECT: OTTER BROOK LAKE

NAME: EXIT BRIDGE

LOCATION: KEENE NH

# BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	YES
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES
a. Piers, abutments with spread footings or short pile foundations.	N/A
<ul> <li>b. Superstructure with simple spans or non- redundant support systems.</li> </ul>	XES
c. Inadequate waterway openings. d. Designs which collect ice and debris.	NO YES
<ul><li>e. All water must pass through or over structure.</li><li>f. Other.</li></ul>	NO N/A
3. Are any characteristics of an aggressive stream or waterway present?	YES
a. Active degradation or aggredation of streambed.	YES
<ul><li>b. Significant lateral movement or erosion of streambanks.</li></ul>	No
<ul><li>c. Steep slopes.</li><li>d. High velocities.</li></ul>	No No
e. Any history of highway or bridge damage during past floods.	No
f. Other.	NA
4. Is the bridge located on a stream reach with any adverse flow characteristics?	No
a. Crossing near stream confluence.	No
<ul> <li>b. Crossing of tributary stream near confluence with larger streams.</li> </ul>	No
<ul><li>c. Crossing on sharp bend in stream.</li><li>d. Location on alluvial fan.</li></ul>	No
e. Other.	NO
5. Other comments or observations.	NO

### NEW ENGLAND DIVISION

	4	
PAGE	*	
FMUL	$\overline{}$	

27 Sept 49	CORPS OF ENGI	NEERS, U.S. ARMY		PAGE
SUBJECT PATING FOR C	MTTEL EPANG	EXIT	BEN DEE	
COMPUTATION				in the first
COMPUTED BY ALL	CHECKED BY	·	DATE	3/2/193
COMPOSED OF THE PROPERTY OF TH	UNLONED OF			

INJEHORY	LOND PATH	Jez		
		HIS	3	352
=6x21.	FLEXURE	73.0	28.2	43.9
	SHEAR	49.3	58.5	106.5
48×21	FLEXULE	18.0	22.1	. 34.4
	546+2	27.8	44.9	ଶ। ସ
CHERATING	LOUD PATRICE			
26 K21	FLEWORE	45.5	55.8	86.9
	SERF	82	97.6	177.5
48 421	FLEAL JAK	32.6	39.9	62.1
	STAFFIEL .	63.0	74.8	134.6

OVERLE HTING	HIS	<b>3</b>	352
INJERSTORY	18.0	22.1	34,4
OPERAT 1126	32.6	39,9	62.1

27 Sept 49

CORPS OF ENGINEERS, U.S. ARMY

SUBJECT OTTER BROOK LAKE ENTRANE EXT BRIDGES

36" × 21" IN HC SLAPS CASE

\_\_\_ CHECKED BY \_\_\_M.D MET

BRIDGE REPLACED 1987

SIMPLE SPAN , PRESTRESSED DECK BEAMS

LENGTH OF SPAN: 43 PEST

TYPE IV-36 (36" x 21" IN HC SLAB)

A = 529.80 in 2

I = 25747 in +

W = 551.9 #/FT

5x = I/c C= 21'/2

=  $\frac{25747 \text{ m}^2}{21 \text{ m}/2}$  =  $\frac{2452.1 \text{ m}^3}{2452.1 \text{ m}^3}$ 

INFORMATION GATHERED FROM TRANSMITTAL NO. 3230-002 RESUBMITTAL 9 OCTOBER 1985

fc' = 5000 PSI

feil = 4000 151

density = 150 PCF

STRANDS

12 - 1/2" \$ 270 K SRLV STRANDS

STRAND AREA = 0.1530 in 2/ STEAND

As= 0.1530 x 12 = 1.8360 in Ast = 0.306 in Ast = 1.53in2

CG OF STRAND = 2" FROM BOTTOM OF BEAM / 3" FROM TOP et - 7.5"

es= 1 - CG = 21" - 2" - 8.5"

CORPS OF ENGINEERS, U.S. ARMY

SUBJECT	DITELLEROOM	- LAKE		<u>, , ,                                </u>	
COMPUTATION	CASE A				1 1 1
	MEI		M.D		8/19/95

INITIAL TENSION ON STRAND = 28.92 KIPS = P.

PB = 28.92 x 10 = 289.2 KIPS (10 STRANDS)

PT = 28.92 x 2 = 57.8 KIPS (2 STRANDS)

INITIAL STRESS ON STRAND Finitial = Pa / As

Finitial = 28.92 / 0.1530 = 189.02 KISI

DESIGN LOADS

DL SELF NT

551.9 #/FF

SUPPLIMENTAL DEAD LOADS

DL BIT TOPPING = (2.5")(150 /12"/(3) = 93.75 = /FT
LL

HIS TRUCK

MDL = (0.5519 E/FT) (43FT)2 = 127.56 FT- K

MSDL = (0.09375 FIFT) (43 FT) = 21.67 FT-K

27 Sept 49

CORPS OF ENGINEERS, U.S. ARMY

OTTER BROOK LAKE

CASE A

MEI

COMPUTED BY -

CHECKED BY M.D

LOSS OF PRESTRESS AASHTO TABLE 9.16.2.2. fe'= 5000 PSI 45,000 PSI PRETENSIONED STRAND (ESTIMATED CALCULATION) funal = 189 - 45 = 144 KSI

Pfinal = 144 ks; (1.836 in²) = 264.38 kips = effective prestress

MP/6 = MOMIENT DUE TO PRESTRESS = PE = (264.38 EIPS) (8.5 in/12) = 187.27 FT-16.1PS

## CALCULATED LOSS OF PRESTRESS

FOIR = CONCRETE STRESS AT COT OF PRESTRESSING STEEL TO PRESTRESSING FONCE AND DEAD LOND OF BEAM IMMEDIATELY AFTEL TRANSFER

Fods = CONCRETE STRESS AT CG OF PRESTRESSING STEEL DUE
TO ALL DEAD LOADS EXCEPT THE DEAD LOADS PRESENT AT THE TIME OF PRESTRESSING

fair = Po + (Poe2) - Mac - (Poe2) Po= 0.9 Pi = 589.54 + 730.39 - 113.65 - 505.34 = 700.94

 $f_{eds} = (21,670)(12)(7.5) = 75.75$ 

USE Comforce GENERATER for = 663 CALCULATED BY DESINE Fels = 59 5-6-85 CORPS OF ENGINEERS, U.S. ARMY

OTTEL BROOK LAKE SUBJECT .

Ches &

ME

COMPUTED BY \_

LOSS OF PRESTLESS

Afs - SH + ES + CRe + CRs

SH= 17000 - 150 RH = 17000 - 150 (70) =

6500 (6.4)

RH = 70 (RELATIVE HUMIDITY)

ES = Es fair = 28000 (663) =

481-2 (6-9)

CRC = 12 (fair) - 7 (faps) = 12 (463) . 7 (59) = 7543 (9-9)

CR3 = 20,000 - 0.4 ES - 0.2 (SH + CR.)

= 20,000 - 0.4 (4842) - 0.2 (6500 + 7543) = 15,255 (9-1.

trand = 189.02-34.14 = 154.88 Ksi

Pfinel = 154.88 (1.836 in2) = 284.36 KIPS

MP/s = MOMENT DUE TO PRESTLESS = Pe

= (284.36 kip) (8.5 in/12) = 201.42

PT = (154.88 KO)(2)(0.1530) = 47.39 KIPS

PB = (154.88) (10) (0.1530) = 236.97 KIPS

MP/s = (47.39) (7.5)/12 = 29.62 FT-EIPS

MMS = (236.97)(3.5)/12 = 167.85 FT- KIPS

CORPS OF ENGINEERS, U.S. ARMY

SUBJECT \_\_\_

OTTEL BROOK GARE

CASE

COMPUTED BY ME' 1

CHECKED BY \_\_\_\_\_ M.D

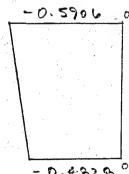
DATE \_ 8/19/73

STRESSES

= 
$$\frac{284.36}{529.8} \pm \frac{29.62(12)}{2452.1} \pm \frac{167.85(12)}{2452.1} \pm \frac{149.23(12)}{2452.1}$$

ALLOWABLE STREESES

STREET AVAILABLE FOR LIVE LOAD



CORPS OF ENGINEERS, U.S. ARMY brook- thief

OTTEL SUBJECT \_\_

CASE

MEI COMPUTED BY \_\_

CHECKED BY M.D

OPERATING MANUAL 5.4.6

FOR PRESTRESSED CONCRETE MEMBERS, THE REINFORCEMENT INVEX DETERMINED BY AASHTO 9.18.1 DOES NOT EXCEED 0.30, THE OPERATING RATING SHALL RESULT IN MOMENTS NOT TO EXCEED 0.75 THE ULTIMATE MOMENT CAPACITY AASHTO 9.17

As \* = 10(0.1530 in 2) = 1.530 in 2

d = h - cg of lower strands = 21" - 2" = 19"

fe' = 5.0 ks1

fs' = 270 KS1

P\* - 4= 1 bd = 1.530 /(36)(19) = 0.0022

P\* - RATIO OF PRESTRESSING STEEL

STEEL STRESSES (BONDED MEMBERS) AASHTO 9.17.4.1 It'su = avg stress in prestressing steel at ultimate load f\*su = fs' (1-0.5 fs' p\* ) = 270 KSI [1-0.5 (0.0022) 270 KSI) f\*su = 253.69 kg1

REILIFORCEMENT INDEX (RECTALGULAR SECTIONS) AASHOO 9.18.1 P\* fsu = 0.0022 (253.69 5 KSI) = 0.1116

0.1116 6 0.30 OK

CORPS OF ENGINEERS, U.S. ARMY

SUBJECT		 ウナブ	FR.	Brest	1-1-	6 6
3000201	-	 				_

COMPUTATION \_\_\_ CASE 4

\_ CHECKED BY \_\_\_\_\_ M.D

CHECK LOCATION OF NEUTRAL AXIS

$$a = \frac{4s^{*} + su^{*}}{0.85 + 6'b} = \frac{(1.53)(253.69)}{0.85 + (5.0)(36)} = 2.54 \text{ in}$$

FLEXURAL STRENGTH

$$M_n = A_5^* f_{5n}^* l (1-0.6 p^* f_{5n}^* / f_c^*)$$
  
=  $(1.53)(253.69)(19/12)[1-0.6 (0.116)] = 573.41 pr-kirs$ 

OPERATING CAFACITY AVAILABLE FOR LIVE COAD

CORPS OF ENGINEERS, U.S. ARMY

SUBJECT \_

CARE

MEI

M.D

8/19/98

LIVE LOND

LOAD FRACTION S/D AASHTO 3.23 4.3

S= WIDTH OF PRECAST MEMBER

C= K(W/L)

C = STIFFNESS PARAMETUR

W = OVERALL WIDTH OF BRIDGE (ft)

L = SPAN LENGTH

C= 0.8 (14 FT/ 43FT) = 0.7605

C 5 5

D= (5.75 - 0.5 NL) + 0.7 NL (1-0.2 C) 1/2 (3-12)

NL = NUMBER OF LANES =

D = (5.75 - 0.5 (1)) + 0.7(1) (1-0.2 (0.2605)) 1/2-

D = 5.93

LOAD FEACTION 5.92 = 0.5055

IMPACT = 50

AASHTO 382.

LIVE LOADS FROM MANUAL PLATE 2

H20: 20/15 (140.95) (1.2976) (0.5058) = 123.35 FT-KIPS

H15: 140.95 (1,2976) (0,5055) = 92.51 FINE

191.75 (1.2976) (0.5058) = 125.85 FT : KIPS

352: 177 35 (12976) (015053) 116.40 57 2185

NED	<b>FORM</b>	223
27 9	ent 4	9

### NEW ENGLAND DIVISION

CORPS OF ENGINEERS, U.S. ARMY

	01
1.4	7/22
PAGE	/ 43

SUBJECT	OTTER	. Brook	CARE				_
COMPUTATION	CASE	1-				7 ·	
COMPUTED BY	MEI	1. 1.	CHECKED BY	M.D	 ATF	3/17/93	

## MOMENT PATHY

TYPE.	INVENDORY (TONS)		OPERATING	(TOMS)
H20	(141.9B)(207) / (23.35 =	23.02	(280.83)(201)	23.35 = 45:
H15			(280.83)(15) 9	
. 11		28.20	(200 83)(25)/12	5.65 - 55.7
352	(14198)(34)/116.40 =	43.91	(250,83)36)/116	.40 = 86.9
,				

CORPS OF ENGINEERS. U.S. ARMY

OTTEL BROOK LAKE

COMPUTATION \_ CKSE A

COMPUTED BY ME !

## CHECK SHEAR + RENFORCEMENT

## CHECK MINIMUM REINFORCEMENT

fr = 7.5 \ fe' = 7.5 \ \( \sigma \) \( \sigma \) \( \frac{4}{2} \) \( \frac{1}{2} \) Mer = fr Sx = 0.53033 (2452.1)/12 = 108.36 1.2 Mer = 1.2 (108.36) = 130.04 FT-KIPS < 573.41 = Mu AASHTO 9-18.2

CHECK SHEAR #4 (CREE 60) @ 15" OC

USE AASHTO 9.20 - THE USE OF 1979 INTERIM METHOD IS ACCORDABLE.

MAX SPACING = 3/4 h = .75(21) = 15.75" >15" OF

Av = (Vu - Ve) 5

fsy = 60,000 psi Assume j = 0.9

Vu-Ve= 2 Av Lay 1'd = 2(0,20)(60)(0,9)(19) = 27.36

Ve=0.06féby d q use smaller b'= 36-2(12) = 12 VL - 180 b'jd

Ye = 0.06 (5000) (12") (0.9) (19") = 61.56 KIPS Ve = 180 (12")(0.9)(19) = 36.94 ELPS CONTROLS

Vn - Ve = 27.36 ) Vn = 27.36 + 36.74 = 64.30 KIPS

DVE = 0.9 (64.30 K) = 57.87 KIPS

ANSHTO 9.14

27 Sept 49

CASE A

MEI

AS PER 1979 INTERIM; CHEEK SHOWN AT 1/4 PT

DL & SDL SHEAR AT 4 POINT

VDL +SDL = (0.5519 EIF + 0.09375 EIF) (43 - 43) = 6.94 K

LIVE LOAD SHEAR AT /4 FOINT L= 43, 0.75L= 32.25, 0.25L= 10.7

IMPACT

impart = 50/125 + 32.25 = 0.3180 .: I = 1.32 CLOSE FLOUGH

MANNIN PLATE 64 7

H20: V= 20(32.25 - 2.8) (1.32) (0.5058) = 9.15 K 90

3: V= 25 (37.25 - 7.44) ( 52) (0.5058) = 9 63 < 95

352: V = 36 (32.25 - 18.61) (1.32) (0.5050) = 762 6 75 MD

USE LOAD FACTOR METHOD

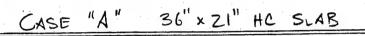
1.3 ( VDL + 5/3 VLL + I ) = + VL = 57.57 CIPS

1. 5/3 VLL+= = +VL - VDL = 57.87 - 6.94 = 37.58 E

(Wellowy: 3/5 (37.50 ) = 22.55 EIPS

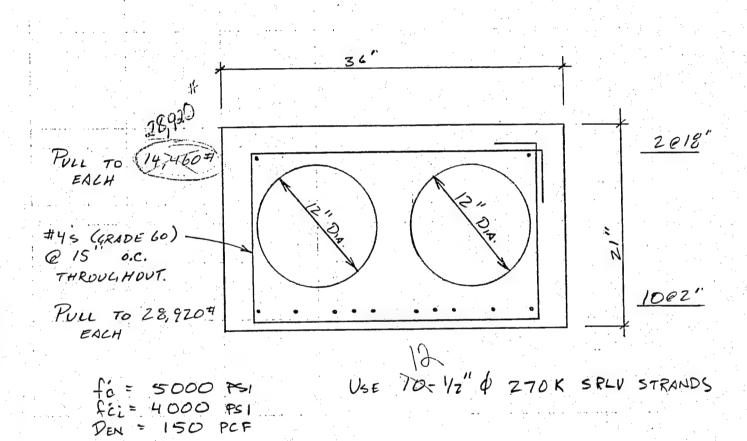
OFERTING: 37.58

TYPE INVENTORY (TONES) OFELLTING (TONG) H 15 (27.55)(20) /9,15 = 49.28 (37.58)(20)/9.15 = 82.19 3 (2255 (25) /9.65 = 58.54 (31.56) (25) /9.63 97.56 3 52 (72.55)(36) /7.62 = 98 106.54 (31.58)(36)/7.62 = 177.54



8-6-85

REVISED



99

CORPS OF ENGINEERS, U.S. ARMY

SUBJECT OTTERBROOK LAKE ENTRANCE/EXIT BRIDGES

COMPUTATION CASE "B" 48 X 21" IN HE SLAB

COMPUTED BY MEI CHECKED BY M.D DATE 5/27/93

BRIDGE REPLACED 1987

SIMPLE SPAN , PRESTRESSED DECL BEAMS

LENGTH OF SPAN : 43 FT

TYPE N-48 (36" x 21" IN HC SLAP)

A = 703.26 in 2 fe' = 5000 PSI

I = 34517 in 4 fai = 4000 psi

w = 732. 6 plf density = 150 pef

5x = 345171/21/3 = 3287.33 in =

FROM TRANSMITTAL NO 3230 - 002 9 OCTOBER 1993

STRAHDS

12 - 1/2" & ZTOE SELV STEANIDS

Ast = 0.1530 x 12 = 1.8260 in 2

As\* = 0.306 in 1

· As\* B = 1,530 in 2

Cg OF STRAND! Z" FROM BOTTOM OF BEAM
3" FROM TOP OF BEAM

es= h - cq = 21" -2" = 8.5"

et = 7.5"

CORPS OF ENGINEERS, U.S. ARMY

OFFICE BROOK CASE

10 15

CHECKED BY -

INITIAL TENSION ON STRAND = P.

Pi = 28.92 Kits

PB = 28.92 × 10 = 289.2 KIPS (10 STERNOS)

PT = 25.92 X 2 = 57.8 KIPS (2 STRAND)

INITIAL STREET ON EA. STRAND

28.92 /0.1580 = 189.02

## DESIGN LOADS

DEAD LONGS

DL Self wh

732.6 plf

SUPPLEMENTAL DEAD LONGS

DL BITUMINOUS SURFACE

CURB

(2.5") (150 pl+)/12 in/c (3') = 93.75 plf. (132 in2)/144 m2/42 (150 plf)=137.5 plf 231.25

LIVE LOND

HIS TRUCK

MOL = (0.7326)(43)2/8 = 169.32 FT-KIPS

MSDLD=(0.09375)(432)/8 = 21.67 FT-KIPS

MSLDQ (0.1375) (43)2/B = 31.78 +T-KIPS

CORPS OF ENGINEERS, U.S. ARMY Orrea Erons SUBJECT \_\_\_\_

1-1-1-1

CACE COMPUTATION -

MEI COMPUTED BY \_\_\_

LOSS OF PRESTRESS AASHTO THOLE 9.16, 2, 2

PRETENSIONED STRAND

fc' = 5000 PSI

for = +15 (from design calculation 8/6/85)

feds = 103 ("

Dfe = SH + ES + CRc + CRc

5H = 17000 - 150 PH = 17000 - 150(70) = 6500 (6-4)

RH = 70

ES = (Es) feir = (28000) 415 = 303 1 (6-9)

CRc = 12 (fer) - 7 (feds) = 12 (415) - 7 (108) = 4224 (9-9)

CRs = 20000 - 0.4 TES - 0.2 (SH+CRC)

= 20000 - 0.4 (3031) - 0.2 (65001 424) =

30,398

final = 189.02 - 30.40 = 158.62 EGI

· Pfinal = 158.62 (1.836) = 291.23 KIPS

M1/6 - Monitaly DUE TO PRESTRESS = Pe

= 291,23 ( 8.5 /12 ) = 206.28 FT-16185

PT = 1158.62)(2)(1530) = 48.54 KIPS

PB = (158.62)(10)(0.1530) = 242.69 FIPS

M 0/5x= (48.54) (7.5/12) = 30.34 FT - KIPS

MI/SB = 242.62 (8.5/12) = 171.90 FT- KIPS

#### NEW ENGLAND DIVISION

PAGE \_ 16/22

CORPS OF ENGINEERS, U.S. ARMY

Direct brook care

CASE B

COMPUTED BY \_\_\_\_ME!

M.D

DATE 8/28/95

STRESSES

= 
$$-\frac{291.23}{703.26}$$
  $\pm \frac{30.34^{'12}}{3287.33}$   $\pm \frac{171.90(12)}{5287.33}$   $\pm \frac{169.32}{3287.33}$   $\pm \frac{21.67}{3287.33}$   $(12)$ 

ALLOWABLE STEESES

STRESS AVAILABLE FOR LIVE LOATS

INVENTORY

CORPS OF ENGINEERS, U.S. ARMY

SUBJECT OTTER BROOK LAKE

DISTATION CASE B

COMPUTED BY

- CHECKED BY

DATE \$ /28 /93

OPERATING MAIJUAL 5.4.6

FOL PRESTRESSED CONCRETE MEMBERS, THE REINFORCEMENT INTOLK DETERMINED BY AASHTO 9.18.1 DOES NOT EXCEED 0.30, THE OFERATING FATING SHALL RESULT IN MOMENTS NOT TO EXCEED 0.75 THE ULTIMATE MOMENT CAPACITY AASHTO 9.17.

As\* = 1.530 in2

d = h - cg of lower strands = 21"-2" = 19"

fe' = 5.0 KS1

fs' = 270 Ks1

p+ = As\*/bd = 1.53 /48)(19) = 0.0017

P# = RATIO OF FRESTEESSING STEEL

STEEL STREESES (BONDERS MEMBERS) AASHTO 9.17.4.1  $f_{su}^* = avg \quad stress in prestressing steel at ult. land$   $f_{su}^* = f_s'(1-0.5) = 270 \left[1-0.5 \left(0.0017\right)(270)\right]$ 

= 257.61 KSI

PRINFORCEMENT INDEX ( RECTANGULAR SECTIONS) ARSTO 9.18.1

PH Fish = 0.0017. (253.69 KSI/5 KSI) = 0.0863

0.0863 6 0.30

## NEW ENGLAND DIVISION CORPS OF ENGINEERS, U.S. ARMY

OFTER BROOK CATE

CHOR B

COMPUTED BY ME I

8/23/23

CHECK LOCATION OF NEUTRAL AXIS  $a = \frac{A_5 * A_5 u^*}{0.85 L' h} = \frac{(1.53)(253.67)}{0.85 (5.0)(40)} = 1.90 \text{ in}$ 

B, = 0.80 AASHTO B.16.2.7

c = a/B, = 1.90 p. 8 = 2375

2.375 < 4.5

OK TO USE RECTANGULAR AREA

FLEXURAL STRENGTH

Mu = As + for & (1-0.6 p\* for /fe' = (1.53) (257.61) (21)/12 [1-0.6 (0.0369)]

= 654.03 - \* NOTE: MU = OM n => O = 1.0 ONLY FOR FACTORS! PRODUCED PRESTRESSED - M.D. AASHTO 9. H.

OPERATION: MEAP = 0.75 (Mu) = 0.75 (654.03) = 490 53 A.

OPERATING CAPACITY K-VAIL HELE FOR LIVE LOAD · Moper = Menp - (Mou + Maor) = 490,53 - (222,77) - 267,76

CORPS OF ENGINEERS, U.S. ARMY

ATTEN BROOK LARET

CASE

115 1

M.D.

## LIVE LOAD

cole (w/L)

SID= LOAD FRACTION

S= WIDTH OF PROCESTANDER

C = STIFFAME PARAMETER

WE OVERHUL BRIDGE WIATH

L = SPAN LENGTH

0.8

C= 0.8 (4/43) = 0.2605

C = 5

. D = (5.75 - 0.5 NL) + 0.7 NL(1-0.2 C) 1/2 (3-12) NL= # OF LANES

D= (5.75 - 0.5) + 0.7[1-0.2(0.2605)] = 5.93

S/D = 4/5.93 = 0.6745

## IMPACT

INPACT: 50/(6+125) = 50/(45+125) = 0.2976

1. I 1.2976

LIVE LOADS FROM MANUAL PLATE Z

. HZO: 20/15 (140.95) (112976) (0,6745) = 164.48 FT- EIFS

(140.95) (12976) (0.6745) = 123.36 FT- KIPS H15:

(111.75) (1,2976) (0,6745) = 157.83 FT-EIPS 3

(177,35) (1.2976) (1.6745) = 155.22 = - E.P. 3521

NE	D	FOF	M	2	23
27	S	Sept	49	9	

## NEW ENGLAND DIVISION CORPS OF ENGINEERS, U.S. ARMY

BROOF LAKE

٠.	201
PAGE	

7778 m COMPUTATION .

ME COMPUTED BY

M.D.

8/28/73

## MOMENT RATING

TYPE	INVENTORY (TONS)	OPERATING (TOMS)	i
		18.05 (267.76)(207)/164.48=	The second second
		18.05 (267.76)(157)/(12336) =	
3	(148.45) (257)/ 167.83=	22.11 (247.76)(257)/167.83 =	39.89
352	(48.45)(3LT)/ 155.22=	3- 43 (148-45 (36T)/155.22 =	34.43

48" FLANK CONTROLS PATING - N.D.

MEI

M.D.

CHECK ENEAL & REINFORCEMENT

fr = 7.5 \(\frac{1}{16'} = 75\)\(\sigma \)\(\sigma = \sigma \)\(\sigma \)\(\sigma \)\(\frac{1}{15}\)\(\sigma \)\(\sigma \)\(\frac{1}{15}\)\(\f

Mer = fr 1x = 0.53033 (3287.33)/12 = 145.28 FT- KIPS

1.2 Mer = 1.2 (145.28) = 174.34 FT- KIPS < 654.03 OK

AASHTO 9.18.2

CHECK SHEAR # 4 (GR 60) @ 15" SE

USE AASHTO 9.20, 1979 INTERIM METHOD IS ACCEPTABLE

MAY SPACHIG = 3/1. h = 0.75 (21) = 15.75 > 15 DE

Av = (Yu-Ve)s fry: 60,000 ps: Assume j = 0.9

Vu- Ve = ZAV fsyjd = Z(0.20)(60)(09)(19) = 27.36

6'= 48-2(12)-10 Ver 0.00 fe b'jol } use smaller Ve = 130 5' d

Vc = 0.06 (5000)(14")(0.9)(19) = 71.82 x1f5

· Vc = 180 (14) (019) (19) = 43.09 KIPS

Val = Ve = 27.36 EIPS Vu = 27.36 + 42.09 = 70.45 MIPS

DVE = 0.9 (70.45 k) = 63.41 kiss

AAGHTO 9.14

CORPS OF ENGINEERS, U.S. ARMY

1151

M.D.

AS PER 1979 INTERIM; CHECK SHEAR AT 1/2 FT

DL + SDL STOR AT /2 FT

VOL+ SOL = (0,732.6+,231.25) (43 - 43) = 10.36 KIFS

LIVE LOAD SHOPE AT 1/4 FOINT

0.75 L = 32.25

MPRET = 50/125 + 32.20 = 0.2130 1. I = 1.32

Not to Enter 1 1 1 30 mil

MAINE PLATE 6 & 7

420: V= 20 (32:35-53 (1.32) (0.6745) = 12.20 k

V= 25 (32.25 -7.011) (182 0.67115)= 12.84 K

V= 36 (32.25.18.61) (1.32) (0.6745) = 10.17 2

DE LOND FACTOR METHOD

3 ( VDL + 53 VLL +1) = \$ 1 VLL = 63.41 EIPS

1 5/3 Num = 6/4 - Vol = 63.4 - 10.36 = 33.42 215

INVENTORY: 3/5 (38.42) = 23.05

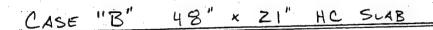
OFERTURE 38.43

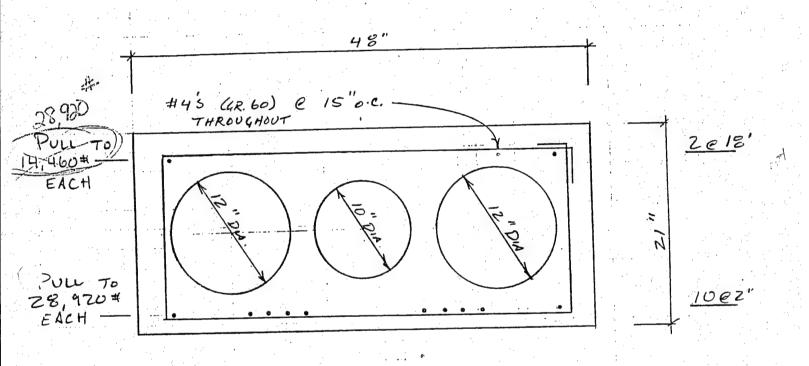
INVESTIGATION Y TYPE (23,05)(20)/12,20 = 37.79 3 (25,05) (25) / 2.84 = +4.83 (3842) (25) / 12.84 = 74.81 2,52 (22,05) (34) /10,4 = 81.54

ofelle Jo (38.42) (20) / 12.20 = 62.96

(38.42 (36) / 1017 - 136.00

ŧ	8-7-85	BESIGN _	LEAP ASSOCIATES INTERNATIONAL INC.			
1	SED	CHECK	JOB FOR	JOB NO. PRC 513		





USE 12- 42" \$ 270 K SRLV STRANDS

# OTTER BROOK LAKE ENTRANCE BRIDGE, KEENE, N.H. FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 18 August 1993

DATE OF PREVIOUS INSPECTIONS: In-depth, \*.

Routine, Sept 87 Routine, Sept 89

Routine, 22 August 1991

#### RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	18.0T 22.1T	32.6T 39.9T	Load rating were recalculated for
352	34.4T	62.1T	prestressed beams.

**EVALUATION** (See attached "Structures Inspection Field Report")

A. Roadway, Railings, and Deck.

The overall condition is good. (condition) 7

The bituminous wearing surface on the deck is in good condition with minor rutting along the wheel lines. There is some minor rutting at the gravel approach on the south side. The bituminous concrete road on the north side has a four inch pothole and some minor rutting along wheel lines. The pavement is also cracked along the slab transition due to one-half inch settlement. The approach guardrails are in very good condition with the exception of a slightly bent end rail on the southeast corner.

- B. Fascia and Curbs
- The overall condition is good (condition 7). Both the curbs and fascias have hairline cracks approximately every two feet. Minor debris and vegetation along the curbing.
- C. Underside of Deck and Bearings.
- The overall condition is good (condition 7). The underside of the deck is in good condition. No problems were observed with the bearings.
- D. Wingwalls and Abutments
- The overall condition is fair (condition 6). In general, the cementitious coating is delaminating and in poor condition.

The abutments appear to be stable. Spalling has occurred on the southeastern wingwall. On the north abutment there is an eight foot by two foot by six inch spall on the northeast corner and a four foot by two foot by six inch spall on the northwest corner. There is also evidence of scour and undermining on the north abutment.

E. Channel

Scour is occurring from four foot deep to the top of the water line on the north abutment with some undermining taking place.

CONDITION RATING

Interim 1987: 7 Interim 1989: 6 Routine 1991: 7 Routine 1993: 7

#### RECOMMENDATIONS

#### Status of Previous Recommendations

	Status of Previous Reco	mmendations	
		Cost E	Est Status
1.	Repair erosion and deteriorated concrete at the base of the abutments and wingwalls.	\$15,00	00 Not Done
2.	Replace bituminous pavement at north approach.	\$3,500	Not Done
3.	Remove all deteriorated concrete repair mortar in wingwalls and abutments and replace with new to give uniform surface.	\$12,50	00 Not Done
4.	Replace nuts on railing post cap.	. Maint.	. Not Done
5.	Remove vegetation from wingwalls and curbs.	\$1000	Not Done
	Tot	al \$32,00	00

#### Revised Recommendations

Implement above recommendations.

Total Updated Estimated Cost \$32,000

## STRUCTURES INSPECTION FIELD REPORT

## **ROUTINE INSPECTION**

city/	KEENE, I	VH		bridge dep		8-structu	re no.	90-date inspecte
2-dist.	104-highway sy		22-owner		27-year		106-year rebuilt	11-milepoint
	ES PESSED	CONCRETE B	RIDGE B	E4m S		ontrol eng	ineer	
07-facility ca	rried EC AREA	ENTLANCE			team le		Cource	
06-features i		EMILIACE			team m	nembers		
OT	TER BROX	DE				FUE	y lorio mark	- DESCHENES
<ol> <li>Dec</li> <li>Stay</li> <li>Curt</li> <li>Mec</li> <li>Side</li> <li>Para</li> <li>Anti</li> <li>Dra</li> <li>Ligh</li> <li>Utili</li> <li>Dec</li> </ol>	dian ewalks apet ing Missile Fence ins ating Standards	7 7 NA 7 NA NA NA NA 7	1. Bearin 2. Stringe 3. Diaphi 4. Girder 5. Floor I 6. Trusse 7. Rivets 8. Welds 9. Collisi 10. Load I 11. Memb 12. Load 13. Paint- 14. Year I	ragms s or Beams Beams es or Bolts on Damage Deflection per Alignmer Vibration Epoxy Painted r Clearance			item 60 SUBSTRUCTURE  1. Abutments a-Wings b-Backware c-Bridge d-Breastare e-Footing f-Piles g-Erosion h-Settlen  2. Piers or Bents a-Caps b-Colum c-Web d-Footing e-Piles f-Scour g-Settler  3. Collision Dama 4. Hydraulic-Aded	all Seats 7 Wall 7 In I
Actual Pos	sting	H 3 3S2	Single	•	Overhe	ead Signs yes	(attached to bridge)	
Recomme From Ratio	nded Posting ng Book		Nr.		1. Wel	ds	NA	
SIGNS IN Y or N	I PLACE	at bridge	advar Na	_	2. Bolt 3. Con		NA	
LEGIBILI	TY	Ā	ولم		Item93	Bb U/W I	nspection Date:	
2. emb 3. fend	i1-channel and nnel scour ankment erosic er system dikes & jetties	on 7 6. effe	ap or slope p	paving Nh 7 7	1. b	raffic Saf ridge railin ansitions pproach g uardrail te	الم إلى guardrail	condition B B B

PROJECT: OFFER BROOK

NAME: ENTRANCE BRIDGE

LOCATION: KEENE, NH

# BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	YES
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructure with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway openings.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	N/A YES NO YES NO NIA
3. Are any characteristics of an aggressive stream or waterway present?	YES
<ul> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	YE > 20 20 20 20 20 20 20 20 20 20 20 20 20
4. Is the bridge located on a stream reach with any adverse flow characteristics?	No
<ul> <li>a. Crossing near stream confluence.</li> <li>b. Crossing of tributary stream near confluence with larger streams.</li> <li>c. Crossing on sharp bend in stream.</li> <li>d. Location on alluvial fan.</li> <li>e. Other.</li> </ul>	70 70 70 70 71 71 71
5. Other comments or observations.	NA

#### NEW ENGLAND DIVISION

CORPS OF ENGINEERS, U.S. ARMY

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PAGE		٠.

_,,	, , , ,	CORPS OF ENGINEERS, C.S. ARMI			•	
UR.IECT	E per 11/4	502-	ETT : 6 6	EPANG	ENSTRANCE	BRUNGE.

CUBJECT FATING FOR STITE FRANK FRANK

COMPUTATION \_\_\_\_\_\_\_ CHECKED BY \_\_\_\_\_\_ DATE 5/2/193

INJENTORY	LOND PATH	2 <i>65</i> HIS	ઢ	352
2121		23.0	28.2	43.9
36x21.	FLEXULE			
	SHEAR	49.3	58.5	106,5
48×21	FLEXULE	18.0	22.1	34.4
	SHEAR	37.8	44.9	81.5
CHERATING	LOUD PARTICIO	-		
36 K21	FLENCURE	45.5	55.8	86.9
	SEAR	82	97.6	177.5
48 KZI	FLETHENER	32.6	39.9	62.1
	2016-62	63.0	74.6	134.6

OVERALL HATIMA
HIS 3 352

INJENTORY 18.0 22.1 34.4

OPERATING 32.6 39.9 62.1

NOTE: FOR CALCULATIONS, SEE EXIT BRIDGE

# COLEBROOK LAKE BRIDGE ON OLD ROUTE 8 SANDISFIELD, MA FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 25 August 1993

<u>DATE OF PREVIOUS INSPECTIONS</u>: In-depth, Dec 84

Routine, Sept 87 Routine, Sept 89 Routine, June 91

#### RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	24T	33T	
3	34T	52T	
3S2	52T	82T	
3-3	60T	98T	

#### EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway, & Railings

The bridge deck, approaches, guardrails, and railings are in very good condition (condition 8). The new deck surface and approaches are still in good condition. There are no visible joints at either end of the bridge. Some of the aggregate from the chip seal surfacing has accumulated along the gutters on the bridge. The cable guardrails along the north approach are both loose. The guardrails along the bridge deck have recently been painted as part of the contract to paint the bridge.

B. Superstructure

The trusses and bracing are in good condition. The entire superstructure has been recently painted. The paint is in good condition, however, the contractor was limited to the amount of scraping that was allowed due to the use of lead in previous coats of paint. This may tend to lead to accelerated degradation of the new finish. All joints, welds, and connections are in good condition. Most deteriorated rivets have been replaced with high strength bolts.

#### C. Underside of Deck

The superstructure under the deck is in good condition. The floor beam connections at the bearings at the ends of the trusses on the inside of the skew angle are filled with sand and painted. Attention will have to be paid to this area in future inspections since it is a likely spot for corrosion. Otherwise they should be cleaned out, filled with concrete, and capped. There is some honeycombing along the underside of the deck. Some remaining burlap was noticed between the floor beams and stringers. The bearings are in good condition.

#### D. Wingwalls and Abutments

The wingwalls and abutments are in good condition. Most vertical cracking has been sealed as recommended in previous inspections. Some horizontal cracking along cold joints in both wingwalls and abutments have not been repaired. Some minor efflorescence was noted along both north and south wingwalls.

#### E. Channel

The channel is in good condition and flowing smoothly. There is a moderate amount of rubble built up in the north side of the channel. The sheetpile and concrete toe protection along the south abutment is in good condition.

#### CONDITION RATING

In-depth 7
Interim 1987 7
Interim 1989 6
Routine 1991 7
Routine 1993 8

#### RECOMMENDATIONS

#### Status of Previous Inspections

Item

- 1. Remove vegetation from southeast wingwall
- 2. Remove vegetation from curb edge.
- 3. Paint structural steel

Not Done Recurring Complete

Status

## Revised Recommendations

Keep the curb edge free of vegetation. No additional recommendations

# STRUCTURES INSPECTION FIELD REPORT

# ROUTINE INSPECTION

City				bridge dep		8-structu		90-date inspected
	DK LAKE	SMADISFIE		COE			JEEMA2510019	25 AUG 93
2-dist.	104-highway syst	em	22-owner		27-year bi	UIIT	106-year rebuilt	11-milepoint
43-structure 07-facility ca	STEEL				team lead	16	FOR-CES COLUCE	
06-features		,			team me	embers		
_FARA	COTahir	RIVER			M. D	ESCHE	NES. M.WALSH	M. IORIO
<ol> <li>Dec</li> <li>Stay</li> <li>Curl</li> <li>Mec</li> <li>Side</li> <li>Para</li> <li>Rail</li> <li>Anti</li> <li>Drai</li> <li>Light</li> <li>Utilit</li> <li>Dec</li> </ol>	dian ewalks apet ing Missile Fence ins ating Standards		<ol> <li>Bearing</li> <li>Stringe</li> <li>Diaphra</li> <li>Girders</li> <li>Floor E</li> <li>Trusse</li> <li>Rivets</li> <li>Welds</li> <li>Collision</li> <li>Load E</li> <li>Member</li> <li>Load V</li> <li>Paint-E</li> <li>Year P</li> </ol>	agms s or Beams seams or Bolts on Damage Deflection er Alignment (ibration Epoxy lainted Clearance —		7 77 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	item 60 SUBSTRUCTURE  1. Abutments	ent —
Actual Pos	ting [	H 3 3S2	Single			yes	attached to bridge)	
From Ratir SIGNS IN Y or N LEGIBILIT	ng Book [	bridge	advan	ce .	Welds     Bolts     Condit	. •		
			Item93b U/W Inspection Date:					
TEM 61-channel and channel protection  annel scour 2. embankment erosion 3. fender system 4. spur dikes & jetties  TEM 61-channel and channel protection  7 5. rip rap or slope paving 7 6. effectiveness 7 7. debris 8. vegetation				1. brid 2. tran 3. app	affic Safe ge railin ssitions croach go rdrail te	uardrail D	condition	

PROJECT: COLETRON LANGE NAME: ALD ROTE & THOSE LOCATION: SANDIN FIELD LIN

# BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	20
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	Ves
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructure with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway openings.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> </ul>	No No No
f. Other. 3. Are any characteristics of an aggressive stream or waterway present?	120
<ul> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	10 10 10 10 10
4. Is the bridge located on a stream reach with any adverse flow characteristics?	<u>x</u> 50
<ul> <li>a. Crossing near stream confluence.</li> <li>b. Crossing of tributary stream near confluence with larger streams.</li> <li>c. Crossing on sharp bend in stream.</li> <li>d. Location on alluvial fan.</li> <li>e. Other.</li> </ul>	1°C 130 13 1
5. Other comments or observations.	Mary and the Property and

#### KNIGHTVILLE DAM INDIAN HOLLOW ROAD BRIDGE, HUNTINGTON, MA FISCAL YEAR 1993 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION: 25 August 93 Routine Inspection, 13 May 91 DATE OF PREVIOUS INSPECTIONS: Inventory Inspection, March 85 RATING (T = TONS)Operating Comments Inventory Type No change in 40T H 17T ratings due to 47T 20T Type 3 inspection findings. Type 3S2 67T 29T (See attached "Structures Inspection Field Report") EVALUATION -Overall condition is good. Superstructure Α. -Both east and west approaches are in -Above Deck fair to good condition. The bituminous pavement at the west approach is unravelling. -There are no bridge railings or approach guardrails. -The wearing surface on the deck is in good condition, with a small amount of sand debris collecting at the curbs. -Overall condition is good. В. Superstructure -The underside of the prestressed -Below Deck concrete planks is in good condition. There are signs of water leakage between the planks near the west abutment. -Overall condition is good. c. Substructure -Both east and west abutments are in good condition. Both have numerous hairline cracks with efflorescence, but this condition is not considered serious. -The channel under the bridge is in fair D. Channel condition, with overgrowth of vegetation, but no signs of scour. 7 Inventory 1985: Overall Numerical E. 7

Condition Rating

Routine 1991:

Routine 1993:

7

## RECOMMENDATIONS

## Status of Previous Recommendations

- Construct a 10' long by 12' wide bituminous approach slab at both approaches.
   A contract is currently underway.
- Construct 25' of approach guardrail at each of the four corners of the bridge.
   \$5,000 Not Done
- Seal cracks in abutments.
   A contract is currently underway.

#### Revised Recommendations

1. Due to the low ADT on Indian Hollow Road, and the low vehicle speeds, it is not recommended to provide approach guardrails. There are no further recommendations at this time.

# STRUCTURES INSPECTION FIELD REPORT

## **ROUTINE INSPECTION**

city/l	Hunting ton	MA		bridge dep	t. no.	8-structu	ure no.  DMA 2510020	90-date inspected 8/25/93
2-dist.	104-highway sys	stem	22-owner Corps of	Eng.	27-year	built	106-year rebuilt	11-milepoint
43-structure type  Prestressed Conc. Slab Simple Span  07-facility carried.				urg.	quality control engineer  Nick Forbes			
- Identif (	- / //	llow Roa	d		team leader Joseph Colveci			
06-features	s intersected  Little R.	ver			team m	lembers	K, Mark Descho	i ene s
2. De 3. Sta 4. Cu 5. Me 6. Sia 7. Pa 8. Ra 9. Ar 10. Dr 11. Lig 12. Ut 13. De	earing Surface eck-Condition ay in Place Forms urbs edian dewalks arapet ailing nti Missile Fence rains ghting Standards	7 8 7 N N N N N N N	1. Bearin 2. Stringe 3. Diaphr 4. Girder 5. Floor E 6. Trusse 7. Rivets 8. Welds 9. Collision 10. Load E 11. Memb 12. Load V 13. Paint- 14. Year F	ragms s or Beams ses or Bolts on Damage Deflection er Alignmen Vibration Epoxy Painted Clearance	t .	7 8 2 2 2 2 3 3 3 1 1 1 1 1 1 1 1	item 60 SUBSTRUCTURE  1. Abutments a-Wings b-Backwa c-Bridge d-Breast e-Footing f-Piles g-Erosion h-Settlen  2. Piers or Bents a-Caps b-Column c-Web d-Footing e-Piles f-Scour g-Settlen  3. Collision Dama 4. Hydraulic-Adeg	all 7 Seats 7 wall 7 gs 7 nent 7
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Knightville Dam Indian Hollow Bridge

#### SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	_ No
2. Is streambed <u>erodible?</u> If so, does the structure have any vulnerable design features?	_Yes_
<ul> <li>a. Piers, abutments with spread footings or short pile foundations.</li> <li>b. Superstructures with simple spans or non-redundant support systems.</li> <li>c. Inadequate waterway opening.</li> <li>d. Designs which collect ice and debris.</li> <li>e. All water must pass through or over structure.</li> <li>f. Other.</li> </ul>	/es
3. Are any characteristics of an aggressive stream or waterway present?	No
<ul> <li>a. Active degradation or aggredation of streambed.</li> <li>b. Significant lateral movement or erosion of streambanks.</li> <li>c. Steep slopes.</li> <li>d. High velocities.</li> <li>e. Any history of highway or bridge damage during past floods.</li> <li>f. Other.</li> </ul>	
4. Is bridge located on stream reach with any adverse flow characteristics?	
<ul> <li>a. Crossing near stream confluence.</li> <li>b. Crossing of tributary stream near confluence with larger streams.</li> <li>c. Crossing on sharp bend in stream.</li> <li>d. Location on alluvial fan.</li> <li>e. Other.</li> </ul>	
5. Other comments or observations.	

## Appendix A

## Visual Assessment for Scour Potential

Everett Lake ---- Choate Brook
Birch Hill ----- Goodnow Road
Old Route 202
Middle Road

GEOTECHNICAL ASSESSMENT

FOR

BRIDGE SCOUR STUDY

 $\mathbf{AT}$ 

CHOATE BROOK BRIDGE

EVERETT RESERVOIR

WEARE, NEW HAMPSHIRE

#### GEOTECHNICAL ASSESSMENT

FOR

BRIDGE SCOUR STUDY

AΤ

CHOATE BROOK BRIDGE

EVERETT RESERVOIR

WEARE, NEW HAMPSHIRE

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#### I. INTRODUCTION

#### 1.1 General

This report presents a visual assessment of scour potential at Choate Brook Bridge which is situated in the reservoir area of Everett Dam. The work was done by Geotechnical Engineering Division as part of the NED Bridge inspection program.

## 1.2 Purpose and Scope

The purpose of the assessment was to obtain information on subsurface and streambed conditions at Choate Brook Bridge and visually evaluate whether there is a potential for scour around the footings and abutments. The scope of work included:

- a. Field reconnaissance of the site during September 1993.
- b. Research of available geological and geotechnical information.
- c. Laboratory testing of streambed samples collected during the September 1993 field reconnaissance of the site.
- d. Report to include locus plan, gradation curve, site description, subsurface and streambed conditions, and assessment.

#### II. SITE CONDITIONS

#### 2.1 Site Location and Description

Everett Dam and reservoir are located along the Piscataquog River, a tributary of the Merrimack River, in south central New Hampshire. Choate Brook is a tributary of the southeasterly flowing Piscataquog River, as shown on the Locus Plan in the Appendix. The bridge is in the northern portion of Everett Lake (recreational pool level) and within one-quarter mile of the normal Piscataquog River channel. Choate Brook has a fairly flat slope in the vicinity of the bridge. It cuts through a relatively flat floodplain. A moderate sloping hill ascends to the west of the bridge. A rough sketch (plan view) of the bridge and adjacent areas is included in the Appendix.

#### 2.2 Bridge Description

Choate Brook Bridge has a concrete slab deck which bears on rubble masonry abutments and footings. A smooth concrete surface has been cast against the west abutment. The abutments and footings appear to be in fair to good condition. Stone revetments protect the corners of the bridge. The outer layer of the revetments are in good condition. However, there does not appear to be filter layers between the outer layer and the subgrade.

The footings of the bridge are founded on sand and gravel. It appears high water velocities have eroded (scoured) the sand and gravel below the south end of the west abutment footing. The void is approximately five feet wide by two feet high and is up to two feet deep. Distress cracks were not noted in the abutment area above the void.

Recently several small repairs have been made to the footings, revetments, and abutments. An apparent void under the north end of the west abutment footing was filled with concrete. Voids between the stones in the top two feet of the east abutment were filled with grout. Voids in the stone revetments at the north end of the bridge were filled with grout. Generally the work looks good except that an area up to three feet wide was not grouted at the junction of the stone revetment and northeast corner of the bridge.

#### 2.3 Site Geology

Choate Brook flows through a low, flat and relatively wide area in the pre-glacial Piscataquog River valley. The valley has been filled with deep glacial outwash deposits and till. The brook has eroded a narrow valley in the outwash deposits and the till. Till and till covered bedrock hills which rise above the lowlands form the perimeter of the brook's drainage area.

#### 2.4 Streambeds and Streambanks

The streambed is slightly meandering. It consists of clean, fine to coarse, sands and gravels with rounded to subangular cobbles and boulders. Gradations for the matrix portion of the streambed are included in the appendix. The cobbles and boulders in the streambed are typically 0.25 to 0.75 feet diameter with a maximum diameter of 1 foot. A beaver dam were observed at the north end of the bridge in the streambed. Water flowed through the dam rather than over the top during the inspection. It is approximately five feet high and 12 feet wide at the base. The water level was approximately 4.5 feet deep upstream of the dam and 2.5 feet deep downstream.

The streambanks are typically fairly low (ten feet or less high) and flat (1 vertical on 3 horizontal to 1 vertical on 10 horizontal). Due to the width of the channel in the vicinity of the bridge, the slopes are not critical. Medium to dense

vegetation grows on the banks.

#### III. ASSESSMENT

## 3.1 Streambed and Streambank Material Characteristics

The streambed materials are deep deposits of hard, durable, rounded to subangular, sands, gravels, cobbles and boulders. The mean diameter, by weight, of the sand to boulder sized materials was visually estimated to be 0.25 to 0.5 feet at Choate Brook Bridge. Laboratory gradation tests (Complete gradation test results are in Appendix.) were performed on samples of the sand and gravel matrix materials that exist between the cobbles and boulders. The results indicate that the mean diameter, by weight, of the streambed matrix materials sampled is 1.5 millimeters (0.06 inches). The mean diameters of the streambed materials could be used in theoretical hydraulic studies to estimate the scour potential around the abutment footings.

#### 3.2 Streambank Materials Characteristics

The streambank matrix material characteristics did not appear to be significantly different than the streambed matrix materials. However, the number and sizes of cobbles and boulders in the streambank materials appeared to be lower than in the streambed materials.

#### 3.3 Scour Potential

High water velocities have scoured the material below the footings at the bridge as described in paragraph 2.2. It appears that high water velocities that occur during future flood events will continue to erode the foundation and the bottom of footing materials. Continued erosion will reduce the bearing capacity of the footings and cause subsequent damage to the superstructure of the bridges at a faster rate than normal weathering.

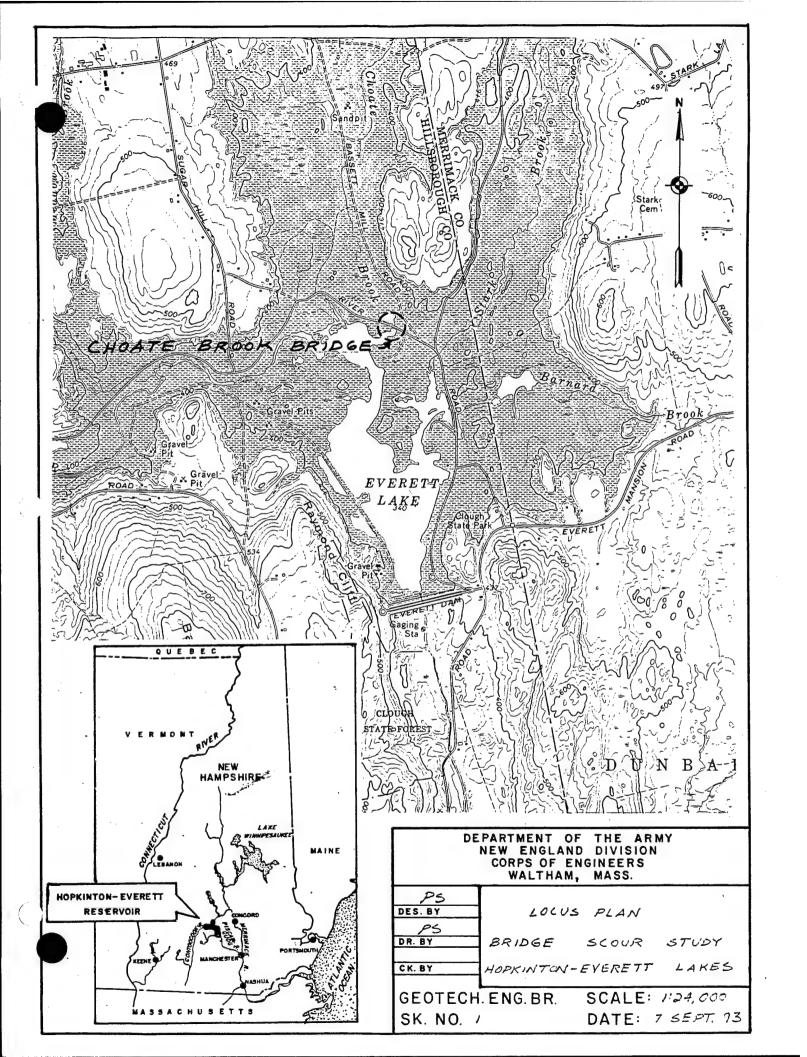
#### 3.4 Proposed Remedial Work at Choate Brook Bridge

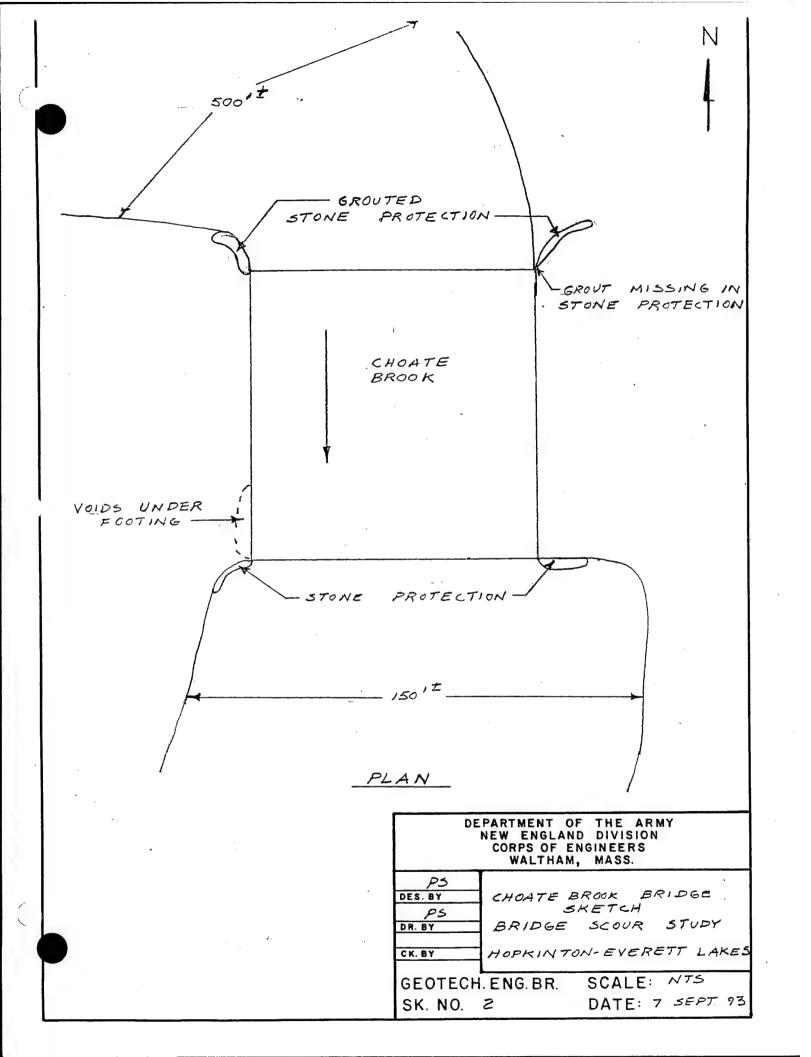
The most pressing need is to fill the void below the west abutment footing. It appears further erosion could damage the abutment. A possible method for repairing the footings is to place concrete forms around the outside edges and then pump concrete into the eroded voids and the space between the footings and forms. Then the entire channel (from approximately 15 feet upstream to 15 feet downstream of the bridge) should be lined with a stone blanket (estimated thickness of 2 feet) underlain by a bedding layer (estimated thickness of 1 foot). The stone blanket and bedding should extend to the top of the banks upstream and downstream of the bridge.

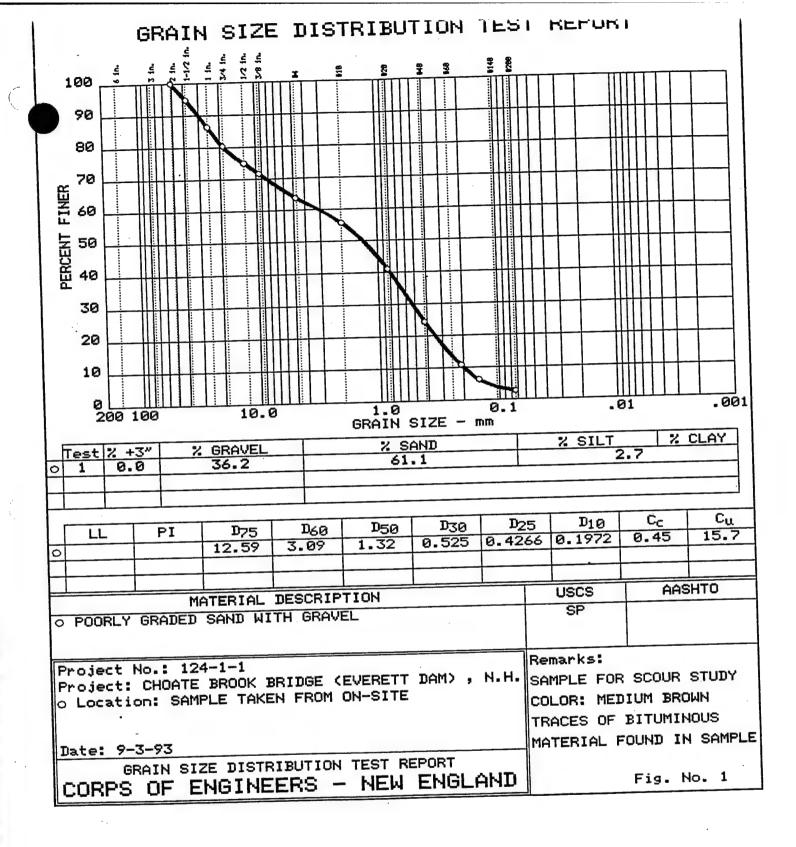
Grout should be placed in the voids of the stone revetment at the junction of the stone revetment and the northeast corner of the bridge. The junction is area of potential future scour because it is weaker than the bridge abutment and grouted stone revetment on either side of it. Approximately one cubic yard of grout and a few hours of hand labor would be required to place the grout.

It is recommended that the beaver dam that was observed upstream of Choate Brook Bridge be removed. It appears that the dam might slightly alter the hydraulic characteristics of the stream and cause eddy currents which could lead to additional scour near the bridge. Rental of a small truck and a few hours of hand labor would be needed to remove the debris.

IV. APPENDIX







GEOTECHNICAL ASSESSMENT

FOR

BRIDGE SCOUR STUDY

AΤ

THREE BRIDGES

BIRCH HILL RESERVOIR

WINCHENDON, MASSACHUSETTS

## GEOTECHNICAL ASSESSMENT

FOR

## BRIDGE SCOUR STUDY

ΤA

## THREE BRIDGES

## BIRCH HILL RESERVOIR

## WINCHENDON, MASSACHUSETTS

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# I. INTRODUCTION

#### 1.1 General

This report presents a visual assessment of scour potential at three bridges situated in the reservoir area of Birch Hill dam. The work was done by Geotechnical Engineering Division as part of the NED Bridge inspection program.

# 1.2 Purpose and Scope

The purpose of the assessment was to obtain information on subsurface and streambed conditions at the three bridges and visually evaluate whether there is a potential for scour around their footings and abutments. The scope of work included:

- a. Field reconnaissances of the sites July 1993.
- b. Research of available geological and geotechnical information.
- c. Laboratory testing of streambed samples collected during a July 1993 field reconnaissance of the sites.
- d. Report to include locus plan, gradation curves, site description, subsurface and streambed conditions, and assessment.

# II. SITE CONDITIONS

# 2.1 Site Location and Description

Birch Hill dam and reservoir are located along the Millers River, a tributary of the Connecticut River, in central Massachusetts. Priest Brook and Beaver Brook are tributaries of the southerly flowing Millers River, as shown on the Locus Plan in the Appendix. The Goodnow Road and Middle Road bridges cross Priest Brook. The Old Route 202 bridge crosses Beaver Brook. The three bridges are within one-half mile of the normal Millers River channel. The tributaries have fairly flat slopes in the vicinity of the three bridges and cut through a relatively flat floodplain. A moderate sloping hill ascends to the north of Old Route 202 bridge. Sketches (plan views) of the bridges and adjacent areas are included in the Appendix.

#### 2.2 Bridge Descriptions

The three bridges have steel girder and concrete decks which bear on concrete abutments and footings except for the Middle Road bridge where the deck bears on chinked stone and mortar abutments and footings. Concrete wingwalls (at each corner) protect Goodnow Road and Old Route 202 bridges while chinked stone and mortar wingwalls (at each corner) protect Middle Road bridge. Gabion extensions have been added to the concrete wingwalls at Goodnow Road bridge.

It appears that the footings for all the bridges are founded on sand and gravel. The footings are in good condition except for the ones at Goodnow Road bridge which have been undermined. It appears high water velocities have eroded (scoured) the sand and gravel below the concrete footings at Goodnow bridge. A steel bar could be pushed from 0.5 to 3.5 feet into nine voids under the south abutment footing and 0.5 to 1 feet into six voids under the north abutment footing. Although voids were observed under the footings at Goodnow Road bridge, no distress cracks other than normal weathering were noted in the abutments.

# 2.3 Site Geology

The Millers River flows through a wide pre-glacial bedrock valley in the vicinity of the three bridges. The valley has been filled with deep glacial outwash deposits of sands and gravels. The river has eroded a narrow inner valley in the sands and gravels which is flanked by sand and gravel terraces. Priest and Beaver Brooks are tributaries that have cut narrow channels through the terraces to the river.

#### 2.4 Streambeds and Streambanks

The streambeds of the two tributaries are slightly meandering.

They consist of clean, fine to coarse, sands and gravels with rounded to subangular cobbles and boulders. Gradations for the matrix portion of the streambed are included in the appendix. The cobbles and boulders in the streambed are typically 1 to 3 feet in diameter with a maximum diameter of 8 feet at the Goodnow Road bridge, typically 0.5 to 1.5 feet in diameter with a maximum diameter of 2 feet at Middle Road bridge and typically 0.25 to 0.75 feet diameter with a maximum diameter of 1 foot at Old Route 202 bridge. Two large boulders (6 to 8 feet in diameter) were observed in the streambed under Goodnow Road bridge. Also a pile (10 by 20 feet) of branches was observed upstream of Goodnow Road bridge. Beaver dams were observed Under Middle Road bridge and approximately 100 feet upstream of the bridge. They were approximately two and three feet high respectively.

The streambanks are typically fairly low (five feet or less high) and steep (1 vertical on 1 horizontal to 1 vertical on 3 horizontal). Medium to dense vegetation grows on the banks. A small amount of erosion of the bank materials at Middle Road bridge was observed. It has occurred 5 to 10 feet upstream of the chinked stone wingwalls. The two eroded areas (scour holes) are 20 to 25 feet long and up to 7 feet wide. It does not appear that the

erosion is endangering the wingwalls or the bridge.

#### III. ASSESSMENT

# 3.1 Streambed and Streambank Material Characteristics

The streambed materials are deep deposits of hard, durable, rounded to subangular, sands, gravels, cobbles and boulders. The mean diameter, by weight, of the sand to boulder sized materials was visually estimated to be 1 to 1.5 feet at the Goodnow Road bridge, 0.5 to 1 feet at the Middle Road bridge and 0.25 to 0.5 feet at the Old Route 202 bridge. Laboratory gradation tests (Complete gradation test results are in Appendix.) were performed on samples of the sand and gravel matrix materials that exist between the cobbles and boulders. The results indicate that the mean diameter, by weight, of the streambed materials sampled is 1.5 millimeters (0.06 inches) at the Goodnow Road bridge, 0.63 millimeters (0.025 inches) at the Middle Road bridge and 10.1 millimeters (0.40 inches) at the Old Route 202 bridge. The mean diameters could be used in theoretical hydraulic studies to estimate the scour potential around the abutment footings.

### 3.2 Streambank Materials Characteristics

The streambank matrix material characteristics did not appear to be significantly different than the streambed matrix materials. However, the number and sizes of cobbles and boulders in the streambank materials appeared to be lower than the streambed materials.

#### 3.3 Scour Potential

High water velocities have scoured material below the footings at Goodnow Road bridge as described in paragraph 2.2. It Appears high water velocities that have occurred during past flood events have not been a problem at Middle Road and Old Route 202 bridges. Field observations and measurements indicate that the top of the streambed is higher adjacent to the abutment footings than at the center of the stream channels at the Middle Road and Old Route 202 bridges. However, high water velocities have eroded streambank materials upstream of Middle Road bridge as described in paragraph 2.4. It appears the erosion of the streambanks near Middle Road bridge is due to the fact that there are fewer and smaller cobbles and boulders in the streambanks as noted in paragraph 3.2.

#### 3.4 Remedial and Maintenance Work at Goodnow Road Bridge

The most pressing need is to fill the voids below Goodnow Road bridge footings. It appears further erosion could damage the abutments. A possible method for repairing the footings is to place concrete forms around the outside edges and then pump concrete into the eroded voids and the space between the footings and forms. Then the entire channel (from approximately 15 feet

upstream to 15 feet downstream of the bridge) should be lined with a stone blanket (estimated thickness of 2 to 3) underlain by a bedding layer (estimated thicknes of 1 to 1.5 feet). The stone blanket and bedding should extend to the top of the banks upstream and downstream of the bridge.

It recommended that the tree debris that was observed upstream of Goodnow bridge be removed. It appears that the debris might slightly alter the hydrologic characteristics of the stream and cause eddy currents which could lead to additional scour near the bridge. Rental of a small truck and a few hours of hand labor would be needed to remove the debris.

#### 3.5 Maintenance Work at Middle Road Bridge

It recommended that the Beaver dam that was observed under Middle Road bridge be removed. It appears that the dam alters the hydrologic characteristics of the stream and causes eddy currents which could lead to scour near the bridge. Rental of a small truck and a few hours of hand labor would be needed to remove the dam.

It is recommended that the erosion upstream of Middle Road bridge be monitored during future bridge inspections. If it appears that the erosion is beginning to endanger the wingwalls, properly designed stone revetments should be constructed to reduce the erosion. They should extend from the streambed to the top of the streambank. The stone revetments are not expected to be major remedial items.

# 3.6 Old Route 202 Bridge

Substantial scour problems were not observed near the Old Route 202 bridge. No remedial or maintenance measures are recommended now at the Old Route 202 bridge.

# IV. APPENDIX

